



# Signal Operations Dodge Street Midtown

Final Report

*Omaha, Nebraska*October 17, 2022





## **Contents**

ĽΧ		Summary	
1	Intro	oduction	3
	1.1	Background	3
	1.2	Signal Locations	3
2	Proj	ect Administration	6
	2.1	Project Team	e
	2.2	Project Meetings	6
3	Data	a Collection	7
	3.1	Existing Traffic Operations Models	7
	3.2	Lane Configurations	7
	3.3	Signal Timing Data	7
	3.4	Detection Data	7
	3.5	24-Hour Week Counts	7
	3.6	Turning Movement Counts	12
	3.7	Travel Time Runs	13
4	Exis	ting Intersection Operations	14
	4.1	Existing Day Plan and Special Plans	14
	4.2	Traffic Observations	14
	4.3	Existing Model Verification	15
5	Imp	lemented Timing Development	17
	5.1	Basic Signal Timing Parameters	17
	5.2	Variable Left-Turn Mode Analysis	17
	5.3	Leading Pedestrian Intervals	18
	5.4	Passage Time Adjustments	20
	5.5	Cycle Length Optimization	20
	5.6	Implemented Day Plan	24
	5.7	Split Optimization	31
	5.8	Offset and Sequence Optimization	33
6	Imp	lementation	36
	6.1	Database Programming	36
	6.2	Implementation Day	36
	6.3	Fine Tuning	36
	6.4	Public Comment	36
7	Perf	ormance Measures	37
	7.1	Network Measures of Effectiveness	37
	7.2	Intersection Operations	40
	7.3	Dodge Street Travel Time Run Performance	41
8	Ben	efit Cost Analysis	44
9	Safe	ty and Operational Recommendations	45
Αŗ	pendix	·	4 <i>€</i>







# **Tables**

Table ES-1. Estimated Network Performance Measures	
Table ES-2. Anticipated Five Year Project Benefits	2
Table 1-1. List of Signalized Project Intersections	3
Table 1-2. Corridor Characteristics	5
Table 3-1. 24-Hour Count Locations and Daily Traffic	7
Table 3-2. Adjusted Peak Hour Segment Volumes During Timing Plan Peak Hours	13
Table 5-1. Updates to Min Green and Min Split Times	17
Table 5-2. Left-Turn Phase Recommendations	18
Table 5-3. Leading Pedestrian Interval Locations and Time	19
Table 5-4. Cycle Length Scenarios for Trial	21
Table 5-5. Plan 1 MD Cycle Length Trial Network Performance	22
Table 5-6. Plan 2 AM Cycle Length Trial Network Performance	22
Table 5-7. Plan 3 PM Cycle Length Trial Network Performance	23
Table 5-8. Plan 4 OP Cycle Length Trial Network Performance	23
Table 5-9. Selected Cycle Lengths for Implementation	24
Table 5-10. Locations with Lagging Left Turn	33
Table 7-1. Corridor and Network Performance Measures	38
Table 7-2. Farnam Street LPI and Recall Sensitivity	39
Table 7-3. Intersection Delay Change Summary	40
Table 7-4. Intersections with Delay Increase Over Five Seconds	40
Table 8-1. Anticipated Five Year Project Benefits	44
Figures	
Figure 1-1. Map of Signalized Project Intersections	
Figure 3-1. 24-Hour Counts on Dodge Street East of 48 <sup>th</sup> Street	
Figure 3-2. 24-Hour Counts on Farnam Street East of 38th Street	
Figure 3-3. 24-Hour Counts on Harney Street East of 35th Street	
Figure 3-4. 24-Hour Counts on Saddle Creek Road north of Farnam Street	
Figure 5-1. Existing and Implemented Weekday Day Plan for Dodge & Farnam	
Figure 5-2. Existing and Implemented Weekday Day Plan for Harney, 42 <sup>nd</sup> , & Saddle Creek	
Figure 5-3. Existing and Implemented Saturday Day Plan for Dodge & Farnam	
Figure 5-4. Existing and Implemented Saturday Day Plan for Harney, 42 <sup>nd</sup> , & Saddle Creek	
Figure 5-5. Existing and Implemented Sunday Day Plan for Dodge & Farnam	
Figure 5-6. Existing and Implemented Sunday Day Plan for Harney, 42 <sup>nd</sup> , & Saddle Creek	
Figure 7-1. Travel Time Run Summary	42







# **EXECUTIVE SUMMARY**

The City of Omaha Public Works Department is updating signal timing plans for different corridors in the city to accommodate current traffic patterns and incorporate safety enhancements. The program grouped various corridors together for different projects to retime the traffic signals. HDR conducted a signal operations study for multiple corridors including Dodge Street in the Midtown area. The objective of this project was to prepare and implement optimized traffic signal timing plans and document measures of effectiveness. Short-term safety and operational improvements were also identified in the study area. In addition, a benefit-cost analysis was conducted for the signal timing optimization.

The Signal Operations project for Dodge Street and the Midtown area retimed 37 existing traffic signals along Dodge Street, Farnam Street, Harney Street, Saddle Creek Road, and 42nd Street. Below is a list of the limits for each corridor that signal timings were updated for:

Dodge Street: 67th Street to 33rd Street

Farnam Street: Dodge Street to 32nd Street

Harney Street: 42nd Street to 33rd Street

Saddle Creek Road: Emile Street to California Street

42nd Street: Emile Street to Dodge Street

Network performance measures were estimated using Synchro models for the existing and implemented signal timings. **Table ES-1** below shows a summary of the existing and implemented network performance measures. Travel time runs were also collected for the Dodge Street corridor for the existing and implemented signal timings using Tru-Traffic software.







**Table ES-1. Estimated Network Performance Measures** 

		Plan 1 - MD		Plan 2 - AM				
	Existing	Implemented	Difference	Existing	Implemented	Difference		
Total Delay (hr)	128	104	-23%	128	140	9%		
Stops (#)	17,338	16,847	-3%	17,111	17,496	2%		
Fuel Consumed (gal)	598	577	-4%	612	620	1%		
Total Travel Time (hr)	441	417	-6%	451	462	2%		
. ,								
		Plan 3 - PM			Plan 4 - OP			
	Existing	Plan 3 - PM Implemented	Difference	Existing	Plan 4 - OP Implemented	Difference		
Total Delay (hr)	Existing 215		Difference -2%	Existing 84		Difference -25%		
Total Delay	, and the second	Implemented			Implemented			
Total Delay (hr)	215	Implemented 211	-2%	84	Implemented 67	-25%		

A benefit-cost analysis was conducted to calculate the benefit-cost ratio and value added by the project. Overall benefits of the project were estimated using a methodology that the City has developed using US Department of Transportation (USDOT) guidelines for monetizing performance measures. Table ES-2 summarizes the benefits of this project.

**Table ES-2. Anticipated Five Year Project Benefits** 

Performance Measure	Project Benefit	Present Value
Delay Reduction	30,295 hours	\$752,835
Fuel Consumption Reduction	47,518 gallons	\$161,474
Emission Reduction	424 tons	\$32,467
Crash Reduction	53 crashes	\$2,893,882

The total cost of the project is not to exceed \$131,972. This results in a benefit-cost ratio of 29:1 over a five-year period.







# 1 INTRODUCTION

#### 1.1 BACKGROUND

The City of Omaha Public Works Department is updating signal timing plans for different corridors in the city to accommodate current traffic patterns and incorporate safety enhancements. The program grouped various corridors together for different projects to retime the traffic signals. HDR conducted a signal operations study for multiple corridors including Dodge Street in the Midtown area. The objective of this project was to prepare and implement optimized traffic signal timing plans and document measures of effectiveness. Short-term safety and operational improvements were also identified in the study area. In addition, a benefit-cost analysis was conducted for the signal timing optimization.

#### 1.2 SIGNAL LOCATIONS

The project included 37 signalized intersections along/near Dodge Street, Farnam Street, Harney Street, 42<sup>nd</sup> Street, and Saddle Creek Road. Study intersections are listed and shown in Table 1-1 and Figure 1-1.

**Table 1-1. List of Signalized Project Intersections** 

Signal ID	Location	Signal ID	Location
279	33 <sup>rd</sup> St & Dodge St	602	36 <sup>th</sup> St & Farnam St
289	35 <sup>th</sup> Ave & Dodge St	1177	38 <sup>th</sup> St & Farnam St
300	38 <sup>th</sup> St & Dodge St	307	40 <sup>th</sup> St & Farnam St
306	40 <sup>th</sup> St & Dodge St	323	42 <sup>nd</sup> St & Farnam St
320	42 <sup>nd</sup> St & Dodge St	340	44 <sup>th</sup> St & Farnam St
339	44 <sup>th</sup> St & Dodge St	76	Saddle Creek Rd & Farnam St
344	46 <sup>th</sup> St & Dodge St	362	50 <sup>th</sup> St & Farnam St
355	49 <sup>th</sup> St & Dodge St	379	52 <sup>nd</sup> St & Farnam St
361	50 <sup>th</sup> St & Dodge St	28	Happy Hollow Blvd & Farnam St
378	52 <sup>nd</sup> St & Dodge St	283	33 <sup>rd</sup> St & Harney St
27	Happy Hollow Blvd & Dodge St	291	36 <sup>th</sup> St & Harney St
1220	Farnam St & Dodge St	309	40 <sup>th</sup> St & Harney St
401	60 <sup>th</sup> St & Dodge St	326	42 <sup>nd</sup> St & Harney St
421	62 <sup>nd</sup> St & Dodge St	319	42 <sup>nd</sup> St & Dewey St
428	66 <sup>th</sup> St & Dodge St	321	42 <sup>nd</sup> St & Emile St
432	67 <sup>th</sup> St & Dodge St	75	Saddle Creek Rd & Emile St
1103	32 <sup>nd</sup> St & Farnam St	74	Saddle Creek Rd & Davenport St
280	33 <sup>rd</sup> St & Farnam St	73	Saddle Creek Rd & California St
1102	34 <sup>th</sup> St & Farnam St		







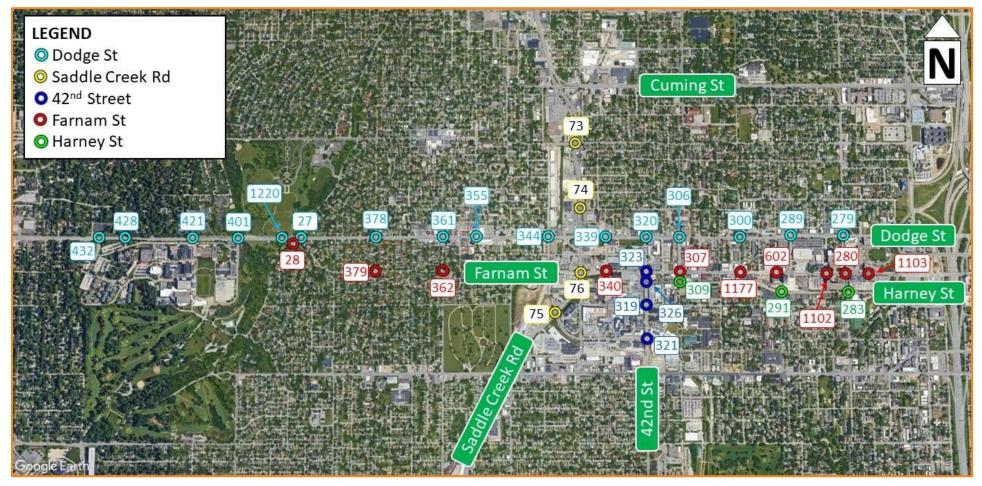


Figure 1-1. Map of Signalized Project Intersections







Table 1-2 details the speed and cross-sectional characteristics of the corridors analyzed for the project.

**Table 1-2. Corridor Characteristics** 

Corridor	Segment	Speed Limit (mph)	Lanes
Dodge St	Turner Blvd – 68 <sup>th</sup> St	35	2 EB / 2 WB / 1 Reversible <sup>1</sup>
Dodge St	68 <sup>th</sup> St – 69 <sup>th</sup> St	35	3 EB / 3 WB
Farnam St	Turner Blvd – 36 <sup>th</sup> St	30	1 EB / 2 WB Angle parking both sides
Farnam St	36 <sup>th</sup> St – 40 <sup>th</sup> St	30	1 EB / 1 WB / 1 Center LT Lane Parallel parking both sides
Farnam St	40 <sup>th</sup> St – 42 <sup>nd</sup> St	30	1 EB / 2 WB Parallel parking south side
Farnam St	42 <sup>nd</sup> St – Saddle Creek Rd	30	2 EB / 2 WB
Farnam St	Saddle Creek Rd – Dodge St	30	1 EB / 1 WB <sup>2</sup>
Harney St	Turner Blvd – Dewey St	30	2 EB Parallel parking both sides Cycle track south side
Harney St	Dewey St – 41 <sup>st</sup> St	30	2 EB Parallel parking south side
Harney St	41 <sup>st</sup> St – Farnam St	30	3 EB
Saddle Creek Rd	Cuming St – Leavenworth St	35	2 NB / 2 SB / 1 Center LT Lane
42nd St	Dodge St – Leavenworth St	30	1 NB / 1 SB / 1 Center LT Lane

 $<sup>^{\, 1}</sup>$  Reversible lane operates EB 6:00–9:00 AM on weekdays and WB the remainder of the time.



 $<sup>^{2}\,</sup>$  Operates as 2 EB lanes 7:00–9:00 AM and 2 WB lanes 4:00–6:00 PM.





# 2 PROJECT ADMINISTRATION

#### 2.1 PROJECT TEAM

The project was guided and completed through efforts from team members comprised from the City of Omaha, Nebraska Department of Transportation (NDOT) and HDR staff listed below.

- Bryan Guy (City of Omaha, Project Manager)
- Jeff Riesselman (City of Omaha, City Traffic Engineer)
- Garret Schram (City of Omaha)
- Nick Gordon (City of Omaha)
- Jenna Habegger (NDOT)
- Mike Forsberg (HDR, Project Manager)
- Kevin Brown (HDR)
- Tahsin Emtenan (HDR)
- Bryce Hallmark (HDR)
- Charlie Fankhauser (HDR)

#### 2.2 PROJECT MEETINGS

Project meetings were held throughout the project to inform team members of activities, get input and make project decisions. Project meetings held are listed below. Meeting minutes can be found in **Appendix**.

- May 2<sup>nd</sup>, 2022 Project Kickoff Meeting
- June 9<sup>th</sup>, 2022 Progress Meeting #1
- June 22<sup>nd</sup>, 2022 Progress Meeting #2
- July 11<sup>th</sup>, 2022 Progress Meeting #3
- July 18<sup>th</sup>, 2022 Pre-Implementation Meeting
- July 27<sup>th</sup>, 2022 Progress Meeting #4
- August 24<sup>th</sup>, 2022 Progress Meeting #5
- October 5<sup>th</sup>, 2022 Progress Meeting #6







# 3 DATA COLLECTION

#### 3.1 EXISTING TRAFFIC OPERATIONS MODELS

Exiting traffic operations models via Synchro were provided by City of Omaha for AM, MD (midday), PM and OP (off-peak) plans.

#### 3.2 LANE CONFIGURATIONS

Lane configurations were collected and verified through use of aerial photography and field observations.

#### 3.3 SIGNAL TIMING DATA

Signal timing data was obtained via remote access to the City of Omaha's MaxView and TrafficView traffic management systems.

#### 3.4 DETECTION DATA

Signal as-builts showing detection and conversation with City of Omaha were used to identify signal detection.

#### 3.5 24-HOUR WEEK COUNTS

24-hour week-long counts were collected at the locations listed in **Table 3-1**. Also shown in the table are the 2018 Metropolitan Area Planning Agency (MAPA) Average Annual Weekday Traffic (AAWT) volumes.

**Table 3-1. 24-Hour Count Locations and Daily Traffic** 

Location	Dates of Counts	Average Weekday Traffic	Average Weekend Day Traffic	2018 MAPA AAWT
Dodge Street east of 48 <sup>th</sup> Street	6/9/22- 6/16/22	36,000	27,600	35,100
Farnam Street east of 38 <sup>th</sup> Street	6/6/22- 6/13-22	8,900	8,400	9,400
Harney Street east of 35 <sup>th</sup> Street	6/6/22- 6/13-22	3,400	2,500	4,400
Saddle Creek Road north of Farnam Street	6/24/22- 7/1/22	27,300	21,500	21,900

Summary of the collected count data is shown in Figure 3-1 through Figure 3-4. This data was used to support the updates to the day plans that define when specific timing patterns operate at each signal. Tables of the data used for these figures can be found in Appendix.







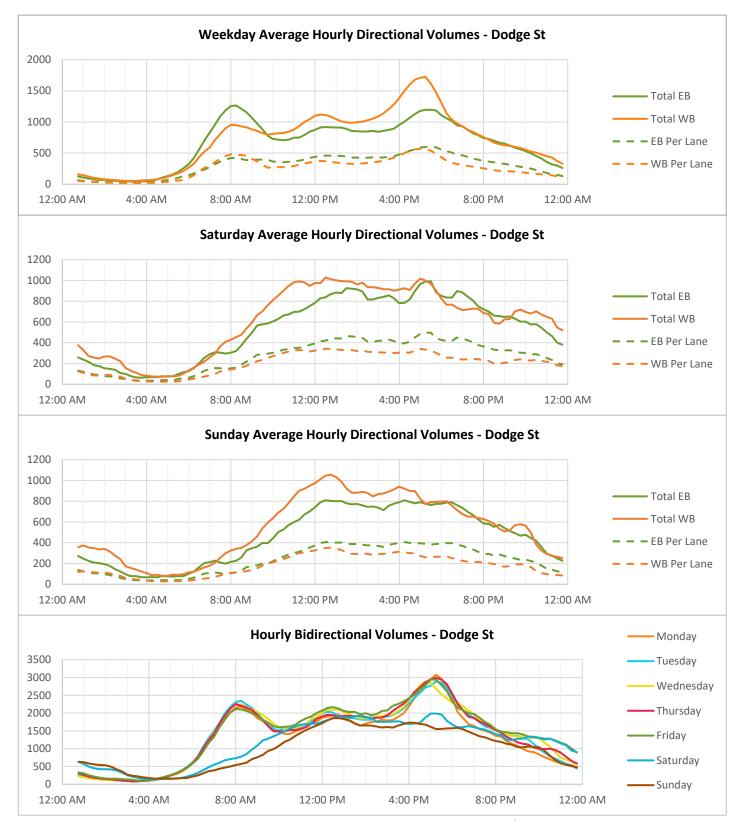


Figure 3-1. 24-Hour Counts on Dodge Street East of 48th Street







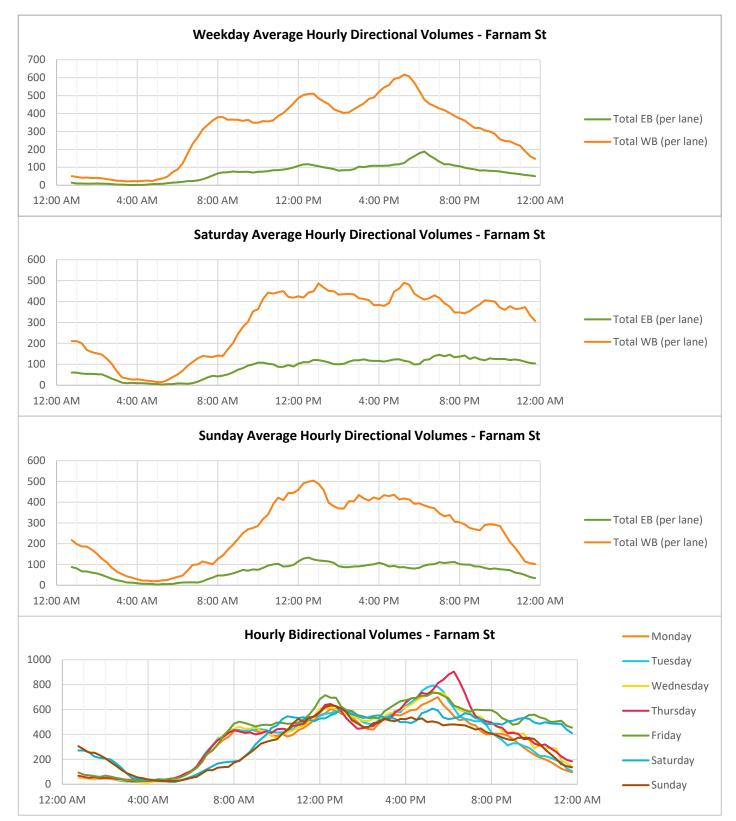


Figure 3-2. 24-Hour Counts on Farnam Street East of 38th Street







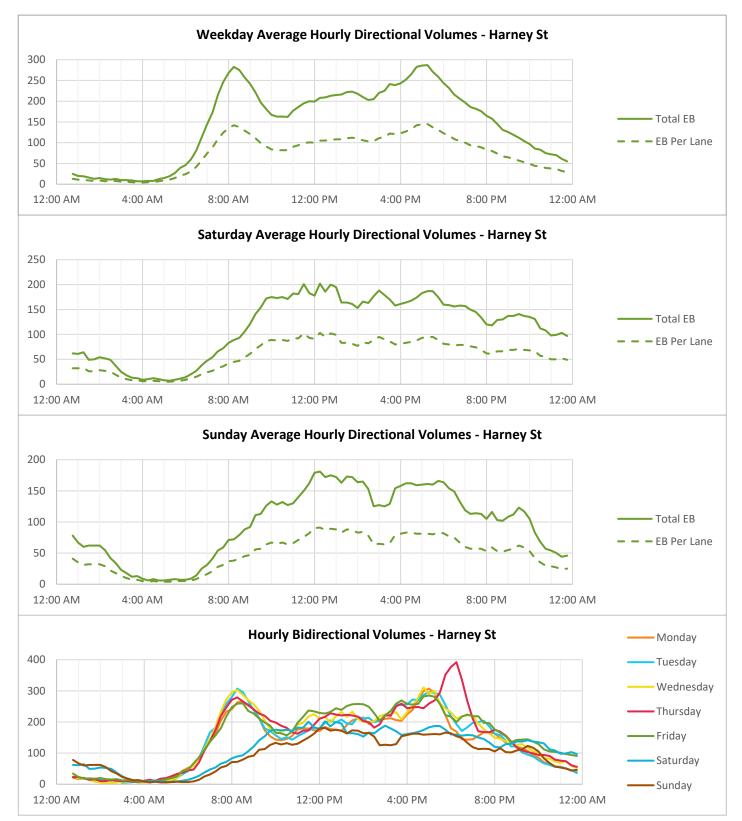


Figure 3-3. 24-Hour Counts on Harney Street East of 35th Street







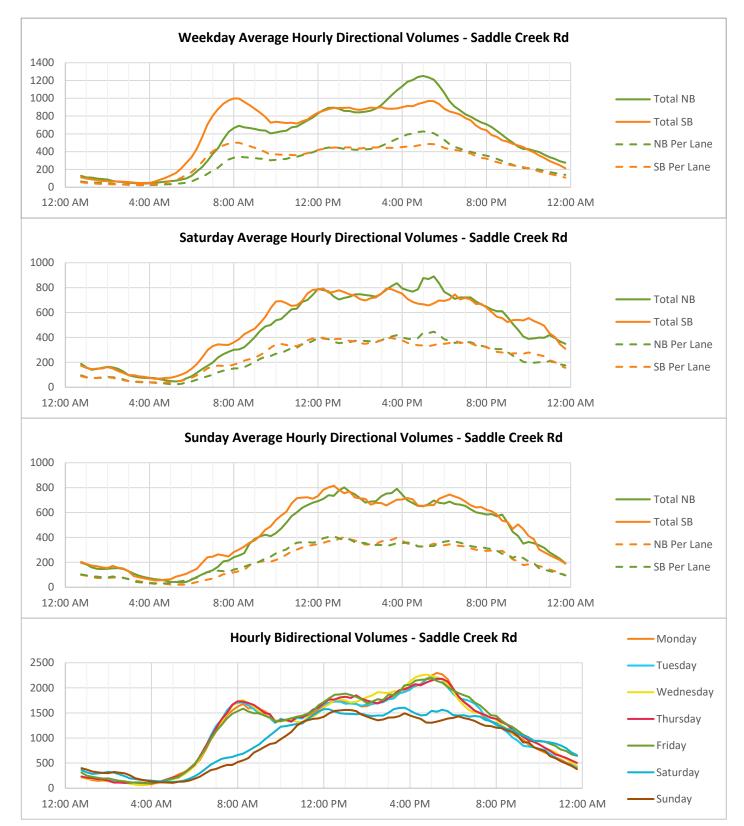


Figure 3-4. 24-Hour Counts on Saddle Creek Road north of Farnam Street







#### 3.6 TURNING MOVEMENT COUNTS

Turning movement counts at study intersections were provided by City of Omaha. Counts were collected across 2019 - 2022. Counts were collected for the 8-hour period of 7 - 11 AM and 2 - 6 PM. Peak hours during signal time plan periods were determined to be:

- Plan 1 MD peak hour: 2:00–3:00 PM; the peak hour is generally over the lunch hour, however, turning movement counts were only available from 2:00–3:00 PM so the 24-hour counts were used to determine adjustment factors to increase the 2:00–3:00 PM volumes to the true peak
  - Dodge Street counts from 2:00–3:00 PM were factored up 6.2%
  - Farnam Street counts west of Saddle Creek Road from 2:00–3:00 PM were factored up 6.2%
  - Farnam Street counts east of 44<sup>th</sup> Street from 2:00–3:00 PM were factored up 22.0%
  - Harney Street counts from 2:00–3:00 PM were factored up 8.4%
  - 44<sup>th</sup> Street & Farnam Street count from 2:00–3:00 PM was factored up 15.2% (average of factors on 42<sup>nd</sup> Street at Farnam Street and Harney Street)
  - 42<sup>nd</sup> Street counts south of Harney Street from 2:00–3:00 PM were factored up 8.4%
  - Saddle Creek Road counts from 2:00–3:00 PM were factored up 0.6%

Plan 2 AM peak hour: 7:30–8:30 AM
Plan 3 PM peak hour: 4:30–5:30 PM
Plan 4 OP peak hour: 9:00–10:00 AM

Adjustments to some counts were made to account for fluctuations in traffic demand over the past few years and better balance traffic volumes throughout the study area. Specifically, in 2019 counts were higher than current traffic demand and in 2020 counts were lower than current traffic demand. 2019 counts were reduced by 20% and 2020 counts were increased by 20%. Then, additions and subtractions to through volumes, generally plus or minus 100-200 vehicles per hour, were made a spot locations to better balance traffic volumes between intersections. Peak hour volumes at spot locations within the study area are shown in Table 3-2.







**Table 3-2. Adjusted Peak Hour Segment Volumes During Timing Plan Peak Hours** 

Location	Direction	Plan 1 MD Peak Hour 2:00-3:00 PM	Plan 2 AM Peak Hour 7:30-8:30 AM	Plan 3 PM Peak Hour 4:30-5:30 PM	Plan 4 OP Peak Hour 9:00-10:00 AM
Dodge Street east of 60 <sup>th</sup> Street	EB	1,118	1,464	1,244	862
Douge Street east of our Street	WB	1,189	1,069	1,801	842
Dodge Street east of FOth Street	EB	958	1,455	1,169	807
Dodge Street east of 50 <sup>th</sup> Street	WB	1,195	906	1,728	826
Dodge Street east of 40th Street	EB	865	1,006	1,054	626
Dodge Street east of 40 <sup>th</sup> Street	WB	927	775	1,233	747
Farnam Street east of 50th Street	EB	173	417		220
rarnam street east of 50 "Street	WB	233		517	86
Farnam Street east of 40 <sup>th</sup> Street	EB	84	75	115	69
Farnam Street east of 40" Street	WB	550	390	631	367
Harney Street east of 40th Street	EB	254	263	299	175
Saddle Creek Road north of	NB	1,038	729	1,420	661
Davenport Street	SB	943	998	875	683
Saddle Creek Road south of	NB	678	496	935	507
Farnam Street	SB	726	918	787	611

#### 3.7 TRAVEL TIME RUNS

Travel time runs were collected along Dodge Street from Turner Boulevard to 69<sup>th</sup> Street for the existing and implemented signal timings. Travel time runs were collected using a GPS device connected to a computer operating TruTraffic v10 software. A minimum of five runs per travel direction were collected during the following periods:

AM Peak: 7:00 – 9:00 AM
 Offpeak: 9:00 – 11:00 AM
 MD Peak: 11:00 AM – 1:00 PM
 PM Peak: 3:30 – 6:00 PM

Weekend MD: 11:00 AM – 1:00 PM

A summary of travel time run data from before and after update to signal timings is provided in the **Performance Measures** chapter of this report. Time-space diagram plots for the existing and implemented for each time period can be found in **Appendix**.







# 4 EXISTING INTERSECTION OPERATIONS

#### 4.1 EXISTING DAY PLAN AND SPECIAL PLANS

For a detailed breakdown of the day plans, the existing are shown side-by-side with the implemented day plans in the Implemented Day Plan section of the Implemented Timing Development chapter.

Only one intersection had a pattern that was not called in the existing day plan.

46<sup>th</sup> St & Dodge St (344) – Pattern 26 (Owl Ride Event Plan, 90 second cycle)

#### 4.2 TRAFFIC OBSERVATIONS

Traffic observations were conducted during the month of June 2022. Observations were made during travel time runs, field visits to the intersections, and virtually through use of the City's camera network. Below is a summary of the observations that were made:

- Happy Hollow Boulevard & Farnam Street Eastbound
  - Eastbound traffic queuing into right eastbound through lane on Dodge Street around 3:00 PM.
- 33rd Street & Farnam Street Eastbound
  - No eastbound left-turn lane. During the PM peak period, there are limited gaps in westbound traffic to allow turns. Waited through multiple cycles waiting for vehicles to turn left.
- 34<sup>th</sup> Street & Farnam Street Westbound
  - Lane utilization is about 80/20 in the two westbound lanes through this section, favoring the right/outside lane.
- 38<sup>th</sup> Street & Farnam Street Westbound
  - Observed long westbound queue extending through 36th Street intersection around noon.
- Farnam East of 42<sup>nd</sup> Street
  - Very little eastbound traffic during the AM period.
- Farnam between Dodge Street and Saddle Creek Road
  - Operates with two-lanes eastbound during the AM peak period and two-lanes westbound during the PM peak period. The remainder of the day it operates two-way with one lane in each direction.
  - It was observed that during the AM peak period, most of the vehicles would stay in the right lane and avoided crossing the yellow dashed centerline. The left lane was used primarily as a left-turn lane or a passing lane to get around slow vehicles. Vehicles would then get back in the right lane. Once the median starts west of Saddle Creek Road, vehicles would split into both eastbound lanes. Lane utilization is about 70/30 in the two eastbound lanes through this section, favoring the right lane.







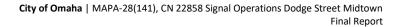
- It operated very similar during the PM peak period. Vehicles traveling westbound at Saddle Creek Road follow the guidance of the "Lane Ends Merge Left" signage and would merge into the left lane. Then once vehicles got to the two-lane section most of the traffic would stay in the right lane and avoided crossing the yellow dashed centerline. The left lane was used almost as a left-turn lane or a passing lane to get around slow traffic. At the intersection with Happy Hollow Boulevard, if the left lane was unoccupied, traffic in the right lane would split into both of the westbound lanes while in the intersection. Lane utilization is about 70/30 in the two westbound lanes through this section, favoring the right lane.
- Both of these conditions create potential conflict points where vehicles merge and diverge on each end of the corridor.
- 42<sup>nd</sup> Street Between Emile Street and Dewey Avenue
  - High pedestrian volume during much of the day slowed traffic at unsignalized pedestrian crossing.
- Farnam Street Through Blackstone District
  - Recent modifications to roadway geometry have had the intended result of slowing vehicular traffic and enhancing pedestrian crossing. Higher pedestrian volume during the MD and PM periods stopped traffic at unsignalized crossing.

#### 4.3 EXISTING MODEL VERIFICATION

Synchro files for the existing AM, MD, OP and PM timing plans were provided by the City of Omaha. The following checks were performed to review and update models to current conditions:

- The files were checked to make sure that the number of intersections matches in the networks.
   49<sup>th</sup> Street & Dodge Street was added to the OP network. 33<sup>rd</sup> Street & Davenport Street and 55<sup>th</sup> Street & Leavenworth Street were removed from MD.
- Signal IDs in the Synchro networks were checked against GIS ID numbers provided by the City.
- The number of lanes on each intersection approach in the Synchro files were checked using Google Earth and field observations. Many updates were made.
- External links lengths were checked for minimum length of 500 feet. Some links were less than 500 feet and updated.
- The approaches at each intersection were checked for cardinal direction.
- Street names were checked to match City of Omaha naming conventions.
- Turn bay storage lengths at the intersections were measured using Google Earth and necessary adjustments were made to the existing storage lengths in the Synchro networks.
- Link speeds in the Synchro files were verified using street view images from Google Earth and field observations.
- Crosswalk widths were set to 16 feet and lane widths were set to 12 feet.
- A growth factor of 1 and a heavy vehicle percentage of 2% were used.









- Detectors in the networks were reviewed and updated at spot locations to match as-builts provided by the City. A detection zone length of 50 feet was used for lanes where the detection zone length was unknown.
- Traffic volumes from turning movement counts were imported into Synchro.
- Base signal timings, cycle lengths, splits and offsets at each study intersection were checked with MaxView and Wapiti and updated in Synchro to match.







# 5 IMPLEMENTED TIMING DEVELOPMENT

#### 5.1 BASIC SIGNAL TIMING PARAMETERS

City staff had recently done a review and update of the Yellow, Red, Walk, and Flash Don't Walk clearance times so they were not reviewed as part of this project.

A review of the existing Minimum Green times and Minimum Splits was conducted for compliance with the City's current guidelines and nature of the intersection approaches. Table 5-1 shows the existing and implemented timings for the Min Green and Min Split.

Table 5-1. Updates to Min Green and Min Split Times

Interesting Name	Signal	Phase/	Min	Green	Min Split		
Intersection Name	ID	Direction	Existing	Implemented	Existing	Implemented	
49 <sup>th</sup> St & Dodge St	355	4/SB	10	8	15	13	
49 St & Douge St	333	8/NB	10	8	15	13	
42 <sup>nd</sup> St & Dewey St	319	4/SB	15	20			
42 St & Dewey St	313	8/NB	15	20			
42 <sup>nd</sup> St & Emile St	321	4 / SB	15	20	20	25	
42 St & Ellille St		8 / NB	15	20	20	25	
	74	2 / EB	10	8	15	13	
Saddle Creek Rd &		4 / SB	15	20	20	25	
Davenport St	/4	6 / WB	10	8	15	13	
		8 / NB	15	20	20	25	
Saddle Creek Rd &	73	2 / EB	9	10	15	16	
California St	/3	6 / WB	9	10	15	16	

### 5.2 VARIABLE LEFT-TURN MODE ANALYSIS

A review and analysis of left-turn movements with existing permitted plus protected (Pm+Pt) phasing was conducted to determine movements to be modified to permitted phasing during specific timing plans. Historical crashes from 2014 to 2020 were reviewed prior to final determination on left-turn phase recommendations and found the only location with left-turn crashes was the southbound, left movement at Saddle Creek Road & California Street with only one crash. This was not enough to warrant a protected left-turn phase. Left-turn phase recommendations by time of day and changes to existing left-turn phase operation are shown in Table 5-2. The left-turn warrant spreadsheets for each movement can be found in Appendix.







**Table 5-2. Left-Turn Phase Recommendations** 

Location - Movement		Plan 1 – MD	Plan 2 – AM	Plan 3 – PM	Plan 4 – OP
Saddle Creek Rd & California St – NB Left	Existing	Pm+Pt	Pm+Pt	Pm+Pt	Pm+Pt
Saudie Creek Ru & Camornia St – NB Leit	Implemented	Perm	Pm+Pt	Pm+Pt	Perm
Saddle Creek Rd & California St – SB Left	Existing	Pm+Pt	Pm+Pt	Pm+Pt	Pm+Pt
Saddle Creek Rd & California St – SB Left	Implemented	Perm	Perm	Perm	Perm
Saddle Creek Rd & Farnam St – EB Left	Existing	Perm	Perm	Perm	Perm
Saddle Creek Rd & Farnam St – EB Left	Implemented	Perm	Perm <sup>1</sup>	Perm	Perm
Saddle Creek Rd & Farnam St – NB Left	Existing	Perm	Perm	Pm+Pt	Pm+Pt
Saddle Creek Rd & Farnam St – NB Left	Implemented	Perm	Perm	Pm+Pt	Perm
Caddle Creek Dd Q Farrage Ct. CD Left	Existing	Pm+Pt	Pm+Pt	Pm+Pt	Pm+Pt
Saddle Creek Rd & Farnam St – SB Left	Implemented	Pm+Pt	Pm+Pt	Pm+Pt	Perm
Caddle Creek Dd Q Freile Ct. CD Left	Existing	Pm+Pt	Pm+Pt	Pm+Pt	Pm+Pt
Saddle Creek Rd & Emile St – SB Left	Implemented	Pm+Pt	Pm+Pt	Perm	Pm+Pt
42 <sup>nd</sup> St & Emile St – NB Left	Existing	Pm+Pt	Pm+Pt	Pm+Pt	Pm+Pt
42 St & Linie St - ND Left	Implemented	Pm+Pt	Pm+Pt	Perm	Pm+Pt

<sup>&</sup>lt;sup>1</sup> Continue to operate Saddle Creek & Farnam EB left as protected during Plan 2 – AM when north leg ped phase is not activated.

#### 5.3 LEADING PEDESTRIAN INTERVALS

A leading pedestrian interval (LPI) is a signal timing adjustment where the walk display is displayed a few seconds prior to the onset of green for the concurrent vehicular movement. The objective is to allow pedestrians to enter the intersection prior to the display of green, making them more visible to turning vehicles during green. LPI usually gives pedestrians 3-7 seconds of a head start entering the intersection, depending on the crossing distance. LPIs have been identified by the Federal Highway Administration (FHWA) as a Proven Safety Countermeasure and have several safety and operational benefits.

Table 5-3 shows the LPI locations and phases implemented with this project based on warrant criteria provided by the City and the corresponding durations in seconds. The LPI warrant spreadsheets for each intersection can be found in Appendix.







**Table 5-3. Leading Pedestrian Interval Locations and Time** 

		Ph	ase 2	Phase 4		Phase 6		Phase 8	
Intersection Name	Signal ID	LPI	Time (sec)	LPI	Time (sec)	LPI	Time (sec)	LPI	Time (sec)
33 <sup>rd</sup> St & Dodge St	279			Х	5			Х	5
35 <sup>th</sup> Ave & Dodge St	289			Х	6			Х	5
38 <sup>th</sup> St & Dodge St	300			Х	5			Х	5
40 <sup>th</sup> St & Dodge St	306			Х	5			Х	5
42 <sup>nd</sup> St & Dodge St	320			Х	5			Х	5
44 <sup>th</sup> St & Dodge St	339			Х	5				
46 <sup>th</sup> St & Dodge St	344			Х	5			Х	5
49 <sup>th</sup> St & Dodge St	355			Х	5			Х	5
50 <sup>th</sup> St & Dodge St	361			Х	5			Х	5
52 <sup>nd</sup> St & Dodge St	378			Х	3			Х	3
60 <sup>th</sup> St & Dodge St	401			Х	6				
62 <sup>nd</sup> St & Dodge St	421			Х	5			Χ	5
66 <sup>th</sup> St & Dodge St	428			Х	5				
67 <sup>th</sup> St & Dodge St	432							Х	5
32 <sup>nd</sup> St & Farnam St	1103	Χ	3	Х	3			Х	3
33 <sup>rd</sup> St & Farnam St	280	Χ	3	Х	3	Х	3	Х	3
34 <sup>th</sup> St & Farnam St	1102			Х	3	Х	3	Х	3
36 <sup>th</sup> St & Farnam St	602	Χ	3	Х	3			Χ	3
38 <sup>th</sup> St & Farnam St	1177	Χ	3	Х	3	Х	3	Х	3
40 <sup>th</sup> St & Farnam St	307	Χ	3	Х	3	Х	3	Х	3
42 <sup>nd</sup> St & Farnam St	323	Χ	3	Х	3	Х	3	Х	3
44 <sup>th</sup> St & Farnam St	340			Х	5			Χ	5
Saddle Creek Rd & Farnam St	76	Х	5			Х	5		
50 <sup>th</sup> St & Farnam St	362	Х	3	Х	3	Х	3	Х	3
52 <sup>nd</sup> St & Farnam St	379	Х	3	Х	3	Х	3	Х	3
33 <sup>rd</sup> St & Harney St	283			Х	3			Х	3
42 <sup>nd</sup> St & Harney St	326	Х	3						
42 <sup>nd</sup> St & Dewey St	319	Х	3	Х	3	Х	3	Х	3
42 <sup>nd</sup> St & Emile St	321	Х	3			Х	3		
Saddle Creek Rd & Emile St	75					Х	5	Х	5







#### **5.4 PASSAGE TIME ADJUSTMENTS**

A review of phase passage time was conducted to determine locations where passage time is not consistent with typical values for detection type/equipment and intersection movement. The following locations were identified and adjusted:

- Saddle Creek Road & Emile Street southbound left-turn phase 7 (radar detection) Updated passage time from 2.2 seconds to 0.7 seconds
- Farnam Street & 34<sup>th</sup> Street southbound approach phase 3 (radar detection) Updated passage time from 3.0 seconds to 1.4 seconds

#### 5.5 CYCLE LENGTH OPTIMIZATION

Multiple cycle lengths were considered at study intersections during each of the four timing plans. Table 5-4 summarizes the combination of cycle length scenarios considered. Additional tables provide network performance metrics for the cycle length scenarios. Selected cycle lengths for implementation are shown in Table 5-9. Performance index is a function of total delay and the number of stops in the analysis area. Overall performance index from the scenario trials were the primary performance metric used to recommend cycle lengths for implementation; however, number of dilemma vehicles and greenband for progression were also used to support selection of cycle length on Dodge Street during the AM plan.







**Table 5-4. Cycle Length Scenarios for Trial** 

	Cycle Length by Corridor or Subarea							
Timing Plan – Scenario (Dodge Corridor Cycle Length)	Dodge St	Farnam St at 50 <sup>th</sup> and 52 <sup>nd</sup> St	Farnam St / Harney St East of 42 <sup>nd</sup> St <sup>1</sup>	Saddle Creek Rd				
Plan 1 MD - Existing	90	60	60	90				
Plan 1 MD – Scenario 1 (60)	60	60	60	90				
Plan 1 MD – Scenario 2 (90)	90	45	60	90				
Plan 1 MD – Scenario 3 (120)	120	60	60	90				
Plan 2 AM - Existing	90	60	60	90				
Plan 2 AM – Scenario 1 (60)	60	60	60	90				
Plan 2 AM – Scenario 2 (90)	90	45	60	90				
Plan 2 AM – Scenario 3 (120)	120	60	60	90				
Plan 3 PM - Existing	90	60	60	90				
Plan 3 PM – Scenario 1 (60)	60	60	60	90				
Plan 3 PM – Scenario 2 (75)	75	75	75	90				
Plan 3 PM – Scenario 3 (90)	90	45	60	90				
Plan 3 PM – Scenario 4 (120)	120	60	60	90				
Plan 4 OP – Scenario 1 (60) <sup>2</sup>	60	60	60	60				
Plan 4 OP – Scenario 2 (90)	90	45	60	90				

<sup>&</sup>lt;sup>1</sup> Includes 42<sup>nd</sup> St & Dewey St and 42<sup>nd</sup> St & Emile St. Includes 44<sup>th</sup> St & Farnam St during Plans 1, 3 and 4 (44<sup>th</sup> St & Farnam St will use Saddle Creek cycle during Plan 2).



 $<sup>^{\</sup>rm 2}\,$  OP plan was new for implementation and did not have an existing set of cycle lengths.





**Table 5-5. Plan 1 MD Cycle Length Trial Network Performance** 

Heading	Existing (90 sec Dodge)	Scenario 1 (60 sec Dodge)	Scenario 2 (90 sec Dodge)	Scenario 3 (120 sec Dodge)	
Control Delay / Veh (s/v)	12	11	11	12	
Total Delay (hr)	323	290	304	325	
Stops / Veh	0.38	0.37	0.36	0.36	
Stops (#)	36,662	35,505	34,062	34,406	
Average Speed (mph)	20	21	21	20	
Total Travel Time (hr)	830	797	811	832	
Fuel Consumed (gal)	1,131	1,100	1,099	1,117	
Unserved Vehicles (#)	65	65	65	65	
Vehicles in dilemma zone (#)	2,483	2,477	2,210	1,813	
Performance Index	424.4	388.2	399	420.3	

Scenario 1 was selected for the MD period since it showed the least amount of overall delay. It also reduced the number of stops compared to the existing. Scenarios 2 and 3 would provide a wider green band for Dodge Street, but the overall delay was higher than Scenario 2 due to side street delay.

**Table 5-6. Plan 2 AM Cycle Length Trial Network Performance** 

Heading	Existing (90 sec Dodge)	Scenario 1 (60 sec Dodge)	Scenario 2 (90 sec Dodge)	Scenario 3 (120 sec Dodge)	
Control Delay / Veh (s/v)	12	11	12	12	
Total Delay (hr)	346	311	330	342	
Stops / Veh	0.37	0.37	0.37	0.36	
Stops (#)	37,779	37,641	37,233	36,363	
Average Speed (mph)	20	21 21		20	
Total Travel Time (hr)	888	853	873	885	
Fuel Consumed (gal)	1,196	1,172	1,181	1,184	
Unserved Vehicles (#)	27	27	27	27	
Vehicles in dilemma zone (#)	2,397	2,370	2,013	1,758	
Performance Index	450.5	415.1	433.2	443.1	

While Scenario 2 did not have the lowest overall delay, it was still selected due to the AM period being more commuter based with heavy eastbound traffic. This allowed for a wider green band than the 60 second cycle for Dodge Street while still reducing the delay compared to the existing. The wider green band also provides some resiliency for future traffic demand as employees continue to transition from working from home.







**Table 5-7. Plan 3 PM Cycle Length Trial Network Performance** 

Heading	Existing (90 sec Dodge)	Scenario 1 (60 sec Dodge)	Scenario 2 (75 sec Dodge)	Scenario 3 (90 sec Dodge)	Scenario 4 (120 sec Dodge)
Control Delay / Veh (s/v)	17	17	16	15	17
Total Delay (hr)	595	595	557	556	597
Stops / Veh	0.47	0.47	0.44	0.43	0.43
Stops (#)	60,339	60,339	56,975	55,336	55,495
Average Speed (mph)	18	18	18	18	18
Total Travel Time (hr)	1,269	1,269	1,231	1,231	1,271
Fuel Consumed (gal)	1,702	1,702	1,651	1,639	1,670
Unserved Vehicles (#)	206	206	206	206	206
Vehicles in dilemma zone (#)	2,907	2,907	3,303	2,783	2,779
Performance Index	762.9	762.9	715.5	710.2	751.2

For the PM period, Scenario 3 was selected since it had the least amount of delay and provided a wide enough green band for the heavier westbound commute from downtown. It also had the least number of stops compared to the other scenarios and the existing.

**Table 5-8. Plan 4 OP Cycle Length Trial Network Performance** 

Heading	Existing (90 sec Dodge)	Scenario 1 (60 sec Dodge)	Scenario 2 (90 sec Dodge)
Control Delay / Veh (s/v)	9	8	9
Total Delay (hr)	184	160	174
Stops / Veh	0.36	0.34	0.33
Stops (#)	25,579	24,613	23,446
Average Speed (mph)	22	23	23
Total Travel Time (hr)	564	540	554
Fuel Consumed (gal)	793	769	770
Unserved Vehicles (#)	0	0	0
Vehicles in dilemma zone (#)	1,883	2,071	1,763
Performance Index	255.2	227.9	239.1







For the OP period, Scenario 1 was selected since a shorter cycle length would provide more responsive signals for side street traffic when Dodge Street traffic is lower. This resulted in a lower overall delay than Scenario 2.

**Table 5-9. Selected Cycle Lengths for Implementation** 

	Cycle Length by Corridor or Subarea									
Timing Plan – Scenario (Dodge Corridor Cycle Length)	Dodge St	Farnam St at 50 <sup>th</sup> and 52 <sup>nd</sup> St	Farnam St / Harney St East of 42 <sup>nd</sup> St <sup>1</sup>	Saddle Creek Rd						
Plan 1 MD - Existing	90	60	60	90						
Plan 1 MD – Scenario 1 (60)	60	60	60	90						
Plan 2 AM - Existing	90	60	60	90						
Plan 2 AM – Scenario 2 (90)	90	60 <sup>2</sup>	60	90						
Plan 3 PM - Existing	90	60	60	90						
Plan 3 PM – Scenario 3 (90)	90	60 <sup>2</sup>	60	90						
Plan 4 OP – Scenario 1 (60) <sup>3</sup>	60	60	60	60						

<sup>&</sup>lt;sup>1</sup> Includes 42<sup>nd</sup> St & Dewey St and 42<sup>nd</sup> St & Emile St. Includes 44<sup>th</sup> St & Farnam St during Plans 1, 3 and 4 (44<sup>th</sup> St & Farnam St will use Saddle Creek cycle during Plan 2).

#### 5.6 IMPLEMENTED DAY PLAN

Implemented day plans for optimized timings are shown alongside the existing in **Figure 5-1** through **Figure 5-6.** Existing and Implemented Sunday Day Plan for Harney, 42nd, & Saddle Creek. The start and end times for each period align with the rising and falling traffic volumes throughout the day as seen in the **24-Hour Week Counts**.

One of the biggest changes to the day plan for the Dodge Street corridor is changing operations from coordinated along Dodge Street to free. This makes the signals more responsive for side street traffic during low traffic times which will now be served a green sooner than they typically would in coordination.

The one intersection with a pattern that was not called in the existing day plan will remain in place per City request for potential future use.

• 46<sup>th</sup> St & Dodge St (344) – Pattern 26 (Owl Ride Event Plan, 90 second cycle)



<sup>&</sup>lt;sup>2</sup> Modified from initial trials of 45 seconds based on added LPI and ped recall which require longer cycle.

<sup>&</sup>lt;sup>3</sup> OP plan was new for implementation and did not have an existing set of cycle lengths.

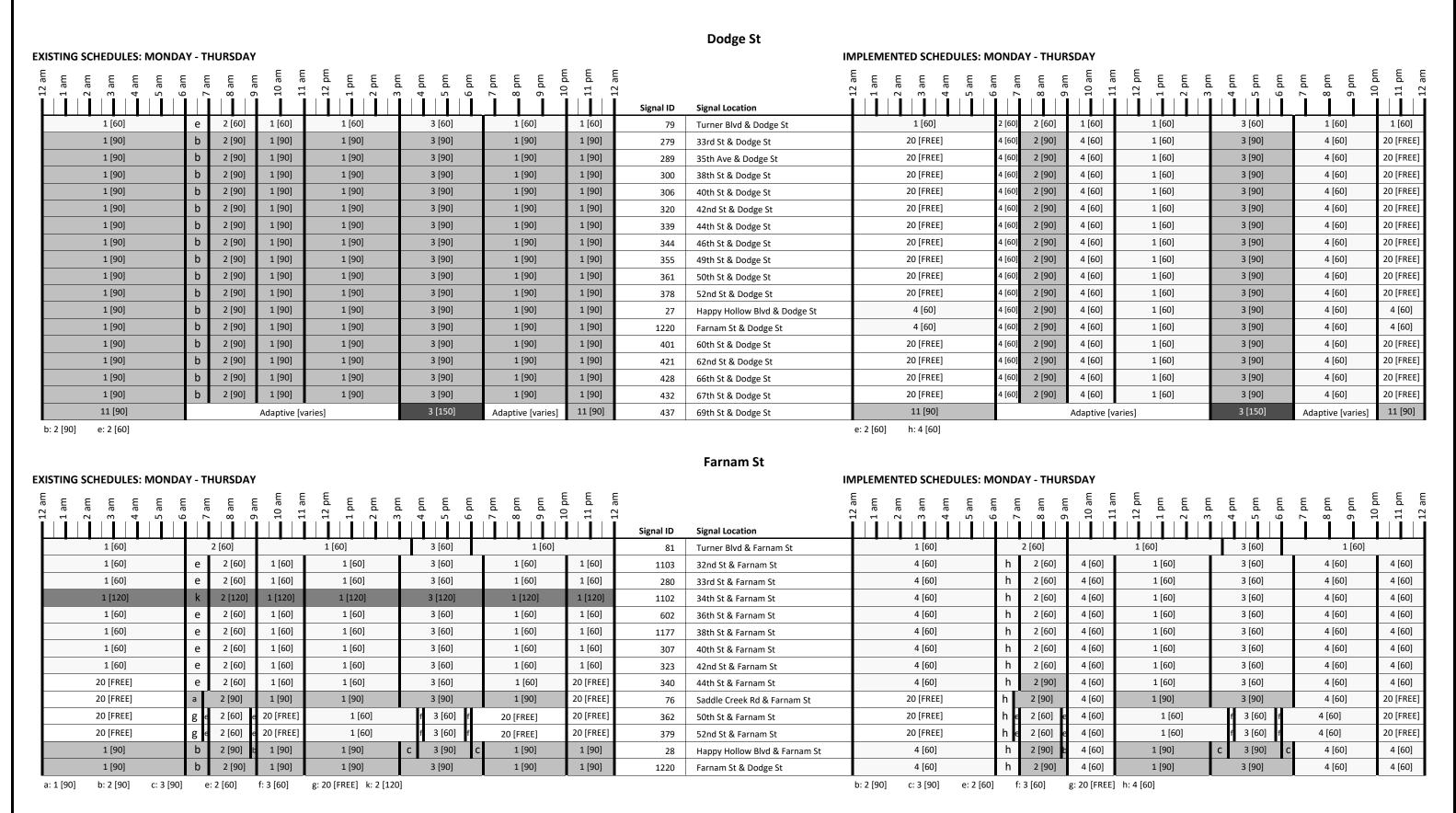
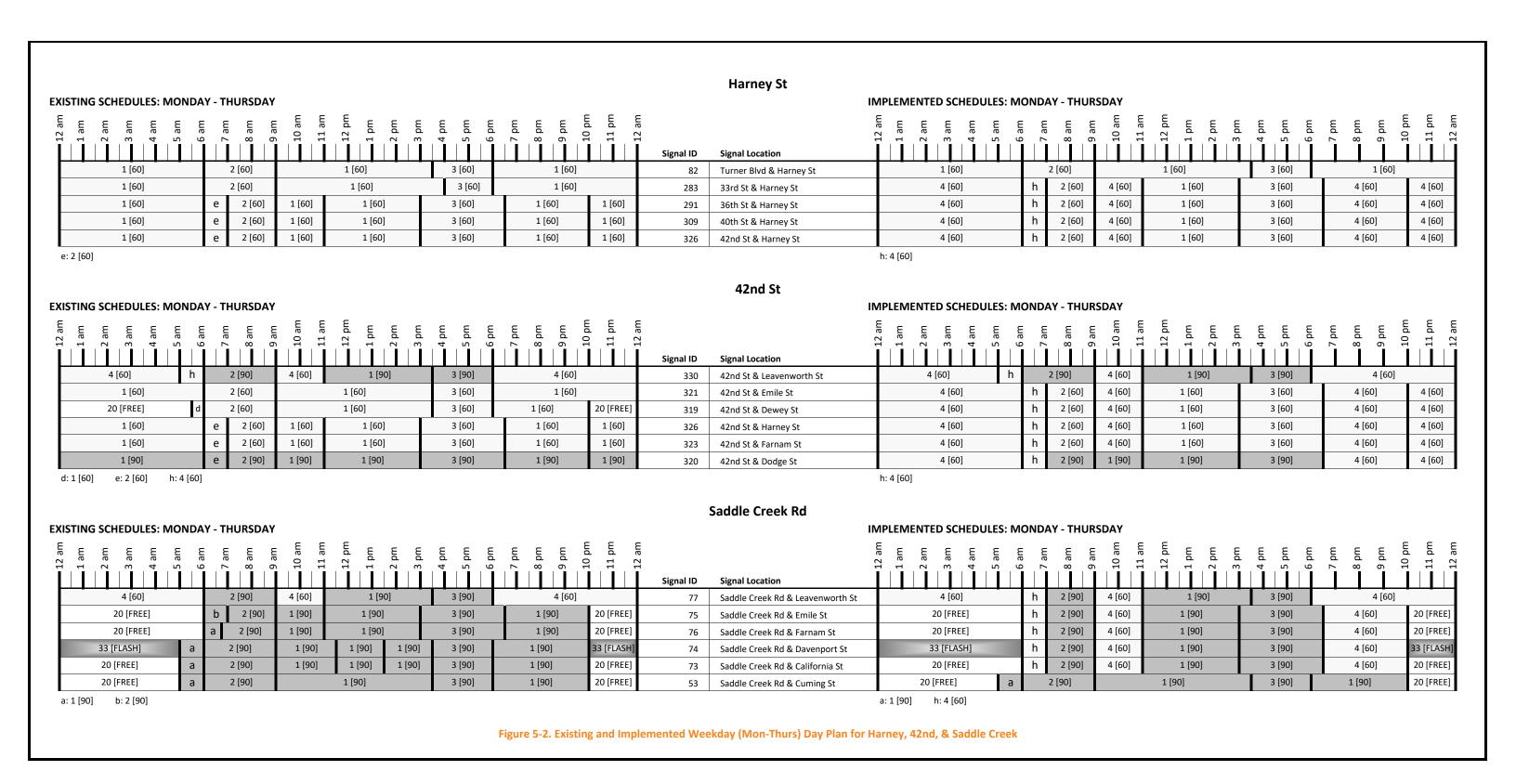


Figure 5-1. Existing and Implemented Weekday (Mon-Thurs) Day Plan for Dodge & Farnam



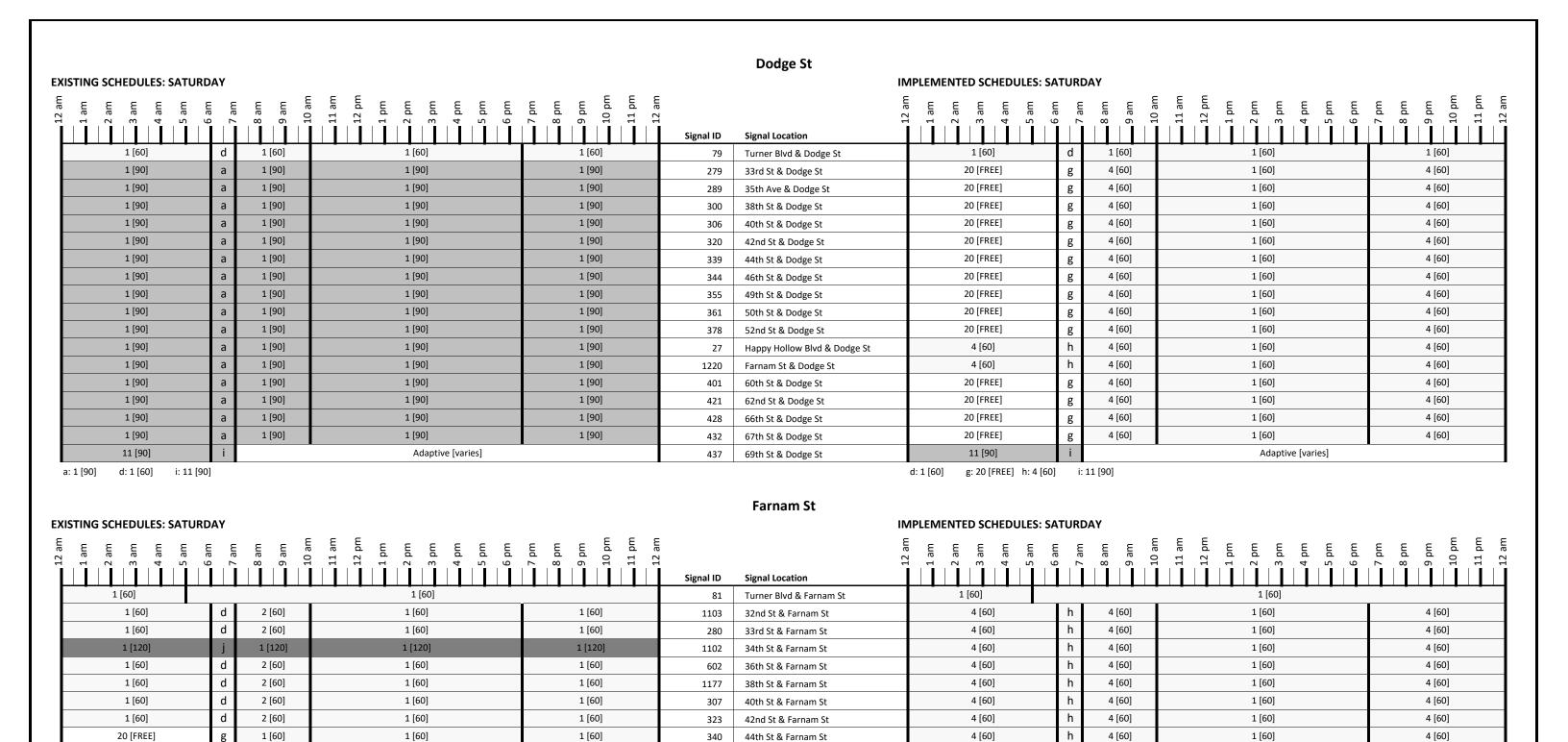


Figure 5-3. Existing and Implemented Saturday Day Plan for Dodge & Farnam

Saddle Creek Rd & Farnam St

Happy Hollow Blvd & Farnam St

50th St & Farnam St

52nd St & Farnam St

Farnam St & Dodge St

76

362

379

28

1220

20 [FREE]

20 [FREE]

20 [FREE]

4 [60]

4 [60]

g: 20 [FREE] h: 4 [60]

4 [60]

4 [60]

4 [60]

4 [60]

4 [60]

1 [90]

1 [60]

1 [60]

1 [60]

1 [60]

4 [60]

4 [60]

4 [60]

4 [60]

4 [60]

20 [FREE]

20 [FREE]

20 [FREE]

1 [90]

1 [90]

d: 1 [60]

a: 1 [90]

g

а

g: 20 [FREE] j: 1 [120]

1 [90]

20 [FREE]

20 [FREE]

1 [90]

1 [90]

1 [90]

1 [60]

1 [60]

1 [90]

1 [90]

1 [90]

20 [FREE]

20 [FREE]

1 [90]

1 [90]

			Harney St					
EXISTING SCHEDULES: SATURDAY				IMPLEMENTED SCHEDULES: S	ATURE	DAY		
2 am am am am bb	9 pm 10 pm 11 pm 12 am			am am am am	an an	E E 8	1 am bm bm bm bm bm bm bm bm bm	
	10 = 10 = 12 = 12 = 12 = 12			12 1 a 1 a 2 a 4 a 4 a 5 a 6 5	_6 a _7 a	8 6 6	6 5 4 4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	7 p
450		Signal ID	Signal Location	4 [60]	Ш		1 [50]	
1 [60] 1 [60] 1 [60]		82	Turner Blvd & Harney St	1 [60] 4 [60]	h	4 [60]	1 [60] 1 [60]	4 [60]
1 [60] d 1 [60] 1 [60]	1 [60]	283	33rd St & Harney St 36th St & Harney St	4 [60]	h	4 [60]	1 [60]	4 [60]
1 [60] d 1 [60] 1 [60]	1 [60]	309	40th St & Harney St	4 [60]	h	4 [60]	1 [60]	4 [60]
1 [60] d 1 [60] 1 [60]	1 [60]	326	42nd St & Harney St	4 [60]	h	4 [60]	1 [60]	4 [60]
d: 1 [60]			·	h: 4 [60]				
			42nd St					
EXISTING SCHEDULES: SATURDAY			II	IMPLEMENTED SCHEDULES: S	ATURD	DAY		
2 am am am am am by	o piri 9 pm 10 pm 11 pm 12 am			2 am am am am	an an	an an	Tam bu	pm pm D pm 1 pm
	9   9   9   9   9   9   9   9   9   9	Signal ID	•	1 1 2 8 4 5	9 7	8 6 7		7 8 6 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
4 [60]		330	Signal Location  42nd St & Leavenworth St	4 [60]			4 [60]	
1 [60]		321	42nd St & Emile St	4 [60]	h	4 [60]	1 [60]	4 [60]
20 [FREE] d 2 [60]		319	42nd St & Dewey St	4 [60]	h	4 [60]	1 [60]	4 [60]
1 [60] d 1 [60] 1 [60]	1 [60]	326	42nd St & Harney St	4 [60]	h	4 [60]	1 [60]	4 [60]
1 [60] d 2 [60] 1 [60]	1 [60]	323	42nd St & Farnam St	4 [60]	h	4 [60]	1 [60]	4 [60]
1 [90] a 1 [90] 1 [90]	1 [90]	320	42nd St & Dodge St	4 [60]	h	4 [60]	1 [60]	4 [60]
d: 1 [60]				h: 4 [60]				
			Saddle Creek Rd					
EXISTING SCHEDULES: SATURDAY		,		IMPLEMENTED SCHEDULES: S	ATURD	DAY		
12 am 12 am 1 am 1 am 2 am 4 am 6 am 10 am 11 am 11 am 12 pm 12 pm 6 pm 7 pm	9 pr 10 p 11 p			12 am 1 am 2 am 3 am 4 am 5 am	6 an 7 an	8 an 9 an	11 am 12 pm 12 pm 2 pm 3 pm 4 pm 5 pm	7 pr 8 pr 9 pr 10 p 11 p
		Signal ID	Signal Location					
4 [60]		77	Saddle Creek Rd & Leavenworth St	4 [60]			4 [60]	
20 [FREE] g 1 [90] 1 [90]	1 [90]	75	Saddle Creek Rd & Emile St	20 [FREE]	g	4 [60]	1 [90]	4 [60]
20 [FREE] g 1 [90] 1 [90]	1 [90]	76	Saddle Creek Rd & Farnam St	20 [FREE]	g	4 [60]	1 [90]	4 [60]
33 [FLASH] a 2 [90] 20 [FREE] a 1 [90]		74	Saddle Creek Rd & Davenport St	33 [FLASH] 20 [FREE]	g	4 [60] 4 [60]	1 [90] 1 [90]	4 [60] 4 [60]
20 [FREE] 1 [90]		73 53	Saddle Creek Rd & California St Saddle Creek Rd & Cuming St	20 [FREE]	g	4 [00]	1 [90]	4 [00]
a: 1 [90] g: 20 [FREE]		J3	Saddle Creek Na & Culling St	g: 20 [FREE]			- [50]	
				<b>5</b> - <b>1</b>				
F	igure 5-4. Existing and I	Implemente	ed Saturday Day Plan for Harney,	, 42nd, & Saddle Creek				

							Dodge St						
<b>EXISTING SCHEDULES: SUNDAY</b>	,							MPLEMENTED SCHEDULES: S	UNDAY				
12 am 1 am 2 am 3 am 4 am 5 am			12 pm 1 pm 2 pm 3 pm 4 pm	2 pm 4 pm 6	10 pm 11 pm 12 am				6 am 7 am 8 am	a 0 t	12 pm 1 pm 2 pm 3 pm 4 pm		10 pm 11 pm 12 am
						Signal ID	Signal Location						
1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	79	Turner Blvd & Dodge St	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	279	33rd St & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	289	35th Ave & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	300	38th St & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	306	40th St & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	320	42nd St & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	339	44th St & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	344	46th St & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	355	49th St & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	361	50th St & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	378	52nd St & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	27	Happy Hollow Blvd & Dodge St	4 [60]	4 [60]	4 [60]	1 [60]	4 [60]	4 [60]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1220	Farnam St & Dodge St	4 [60]	4 [60]	4 [60]	1 [60]	4 [60]	4 [60]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	401	60th St & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	421	62nd St & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	428	66th St & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	432	67th St & Dodge St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
11 [90]	11 [90]		Adaptive [varies]		11 [90]	437	69th St & Dodge St	11 [90]	11 [90]		Adaptive [varies]		11 [90]

EXISTING SCI	HEDULES: SUNDA	Υ						II	MPLEMENTED SCHEDULES: SI	UNDAY				
12 am 1 am 2 am	3 am 5 am	o am 7 am 8 am	9 am 10 am 11 am	12 pm 1 pm 2 pm 3 pm 4 pm	6 pm 7 pm 8 pm 9 pm	11 pm 12 am		;	1 am 2 am 3 am 5 am	6 am 7 am 8 am	9 am 10 am	12 pm 1 pm 2 pm 3 pm 4 pm	6 pm 7 pm 8 pm 9 pm	10 pm 11 pm 12 am
							Signal ID	Signal Location						
1	L [60]			1 [60]			81	Turner Blvd & Farnam St	1 [60]			1 [60]		
	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	1103	32nd St & Farnam St	4 [60]	4 [60]	4 [60]	1 [60]	4 [60]	4 [60]
	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	280	33rd St & Farnam St	4 [60]	4 [60]	4 [60]	1 [60]	4 [60]	4 [60]
	1 [120]	1 [120]	1 [120]	1 [120]	1 [120]	1 [120]	1102	34th St & Farnam St	4 [60]	4 [60]	4 [60]	1 [60]	4 [60]	4 [60]
	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	602	36th St & Farnam St	4 [60]	4 [60]	4 [60]	1 [60]	4 [60]	4 [60]
	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	1177	38th St & Farnam St	4 [60]	4 [60]	4 [60]	1 [60]	4 [60]	4 [60]
	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	307	40th St & Farnam St	4 [60]	4 [60]	4 [60]	1 [60]	4 [60]	4 [60]
	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	1 [60]	323	42nd St & Farnam St	4 [60]	4 [60]	4 [60]	1 [60]	4 [60]	4 [60]
	20 [FREE]	20 [FREE]	1 [60]	1 [60]	1 [60]	20 [FREE]	340	44th St & Farnam St	4 [60]	4 [60]	4 [60]	1 [60]	4 [60]	4 [60]
	20 [FREE]	20 [FREE]	1 [90]	1 [90]	1 [90]	20 [FREE]	76	Saddle Creek Rd & Farnam St	20 [FREE]	20 [FREE]	4 [60]	1 [90]	4 [60]	20 [FREE]
	20 [FREE]	20 [FREE]	20 [FREE]	1 [60]	20 [FREE]	20 [FREE]	362	50th St & Farnam St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
	20 [FREE]	20 [FREE]	20 [FREE]	1 [60]	20 [FREE]	20 [FREE]	379	52nd St & Farnam St	20 [FREE]	20 [FREE]	4 [60]	1 [60]	4 [60]	20 [FREE]
	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	28	Happy Hollow Blvd & Farnam St	4 [60]	4 [60]	4 [60]	1 [60]	4 [60]	4 [60]
	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1 [90]	1220	Farnam St & Dodge St	4 [60]	4 [60]	4 [60]	1 [60]	4 [60]	4 [60]

Farnam St

Figure 5-5. Existing and Implemented Sunday Day Plan for Dodge & Farnam

#### **Harney St EXISTING SCHEDULES: SUNDAY IMPLEMENTED SCHEDULES: SUNDAY** \_\_\_ 12 pm \_\_\_11 pm 12 am \_\_ 10 am Signal ID **Signal Location** 1 [60] 1 [60] 82 Turner Blvd & Harney St 1 [60] 1 [60] 1 [60] 1 [60] 1 [60] 283 4 [60] 4 [60] 4 [60] 1 [60] 4 [60] 4 [60] 33rd St & Harney St 1 [60] 1 [60] 4 [60] 4 [60] 1 [60] 1 [60] 1 [60] 1 [60] 291 4 [60] 4 [60] 1 [60] 4 [60] 36th St & Harney St 1 [60] 1 [60] 4 [60] 4 [60] 1 [60] 1 [60] 1 [60] 1 [60] 4 [60] 4 [60] 4 [60] 1 [60] 309 40th St & Harney St 1 [60] 1 [60] 1 [60] 1 [60] 4 [60] 4 [60] 4 [60] 1 [60] 1 [60] 326 42nd St & Harney St 4 [60] 4 [60] 1 [60] 42nd St **EXISTING SCHEDULES: SUNDAY IMPLEMENTED SCHEDULES: SUNDAY** 11 pm .10 am 1 pm \_\_3 pm 4 pm \_\_3 pm Signal ID Signal Location 4 [60] 4 [60] 4 [60] 4 [60] 330 42nd St & Leavenworth St 1 [60] 4 [60] 4 [60] 4 [60] 1 [60] 42nd St & Emile St 4 [60] 1 [60] 4 [60] 4 [60] 20 [FREE] 2 [60] 20 [FREE] 4 [60] 4 [60] 4 [60] 1 [60] 4 [60] 319 42nd St & Dewey St 1 [60] 1 [60] 1 [60] 1 [60] 1 [60] 4 [60] 4 [60] 4 [60] 1 [60] 4 [60] 4 [60] 1 [60] 326 42nd St & Harney St 1 [60] 1 [60] 1 [60] 1 [60] 1 [60] 1 [60] 4 [60] 4 [60] 4 [60] 1 [60] 4 [60] 4 [60] 323 42nd St & Farnam St 1 [90] 1 [90] 4 [60] 1 [90] 1 [90] 1 [90] 1 [90] 4 [60] 4 [60] 4 [60] 320 42nd St & Dodge St 4 [60] 1 [60] d: 1 [60] Saddle Creek Rd **IMPLEMENTED SCHEDULES: SUNDAY EXISTING SCHEDULES: SUNDAY** 11 pm 12 pm 12 am 11 am 12 pm 11 pm 10 am 10 am .1 pm **-** 1 pm \_\_2 pm 8 am \_\_2 pm \_\_3 pm \_\_8 pm 9 pm 8 am \_\_3 pm .6 pm \_\_7 am рш Signal ID Signal Location 4 [60] 4 [60] 4 [60] 4 [60] 77 Saddle Creek Rd & Leavenworth St 20 [FREE] 20 [FREE 20 [FREE] 20 [FREE] 20 [FREE] 1 [90] 1 [90] 1 [90] 75 Saddle Creek Rd & Emile St 20 [FREE] 4 [60] 1 [90] 4 [60] 20 [FREE] 20 [FREE] 1 [90] 1 [90] 1 [90] 20 [FREE] 20 [FREE] 20 [FREE] 76 20 [FREE] 4 [60] 1 [90] 4 [60] Saddle Creek Rd & Farnam St 33 [FLASH] 2 [90] 33 [FLASH 20 [FREE 4 [60] 33 [FLASH] 74 33 [FLASH] 4 [60] 1 [90] Saddle Creek Rd & Davenport St 1 [90] 20 [FREE] 20 [FREE] 20 [FREE] 20 [FREE] 20 [FREE] 4 [60] 1 [90] 4 [60] 73 Saddle Creek Rd & California St 1 [90] 20 [FREE] 20 [FREE] 1 [90] 20 [FREE] 20 [FREE] 53 Saddle Creek Rd & Cuming St a: 1 [90] Figure 5-6. Existing and Implemented Sunday Day Plan for Harney, 42nd, & Saddle Creek





#### 5.7 SPLIT OPTIMIZATION

Prior to optimizing split times, the following updates were made in Synchro models that coincided with timing changes:

- Add Pm+Pt left-turn phasing for southbound left at Saddle Creek Rd & Davenport St during Plan 3
   PM.
- Add pedestrian recall for the following locations to coincide with LPIs:
  - 32<sup>nd</sup> St & Farnam St Phases 4/8 (NB/SB)
  - 33<sup>rd</sup> St & Farnam St Phases 4/8 (NB/SB)
  - 34<sup>th</sup> St & Farnam St Phases 4/8 (NB/SB)
  - 50<sup>th</sup> St & Farnam St Phases 4/8 (NB/SB)
  - 52<sup>nd</sup> St & Farnam St Phase 4 (NB/SB)
- Add 3 seconds of lost time to the following locations with recall and LPI:
  - 32<sup>nd</sup> St & Farnam St Phases 2/6/8 (all vehicular phases)
  - 33<sup>rd</sup> St & Farnam St All phases
  - 34<sup>th</sup> St & Farnam St Phases 2/4/6 (all vehicular phases)
  - 36<sup>th</sup> St & Farnam St Phases 2/6/8 (all vehicular phases)
  - 38<sup>th</sup> St & Farnam St All phases
  - 40<sup>th</sup> St & Farnam St All phases
  - 42<sup>nd</sup> St & Farnam St All phases
  - 50<sup>th</sup> St & Farnam St All phases
  - 52<sup>nd</sup> St & Farnam St All phases
  - 33<sup>rd</sup> St & Harney St Phases 4/8 (NB/SB)
  - 42<sup>nd</sup> St & Harney St Phase 2 (EB)
  - 42<sup>nd</sup> St & Dewey St All phases
  - 42<sup>nd</sup> St & Emile St Phases 2/6 (EB/WB)

Split times were updated to meet current vehicle demand and account for pedestrian signal timings. Adjustments to transit signal priority (TSP) timings on the Dodge Street corridor were also made to reflect the updated split times. The following provides detailed information by plan.

#### 5.7.1 PLAN 1 MD SPLIT OPTIMIZATION

Split times for minor road through phases at Dodge Street intersections were set to accommodate pedestrian crossing times to prevent signals from going into transition following a pedestrian actuation.

Split times on Farnam Street, Harney Street and 42<sup>nd</sup> Street accommodate pedestrian crossing times via recall.







#### 5.7.1.1 DODGE STREET & FARNAM STREET & HAPPY HOLLOW BOULEVARD

Using a 60-second cycle at Dodge Street & Happy Hollow Boulevard during Plan 1 MD required a lower pedestrian walk of 7 seconds (reduced from 10 seconds) for Phase 4 to allow the split to accommodate the pedestrian time.

Using a 60-second cycle at Farnam Street & Happy Hollow Boulevard during Plan 1 MD required a lower pedestrian walk of 7 seconds (reduced from 11 seconds) for Phase 2 and 6 seconds (reduced from 10 seconds) for Phase 8 to allow the split to accommodate the pedestrian time.

#### 5.7.2 PLAN 2 AM SPLIT OPTIMIZATION

Split times for minor road through phases at Dodge Street and Saddle Creek Road intersections were set to accommodate pedestrian crossing times based mostly on a relatively high number of pedestrians crossing in the peak hour (8 or more) and to prevent signals on these corridors from going into transition following a pedestrian actuation.

Split times on Farnam Street, Harney Street and 42<sup>nd</sup> Street accommodate pedestrian crossing times via recall.

#### 5.7.3 PLAN 3 PM SPLIT OPTIMIZATION

Split times for minor road through phases at Dodge Street and Saddle Creek Road intersections were set to accommodate pedestrian crossing times based mostly on a relatively high number of pedestrians crossing in the peak hour (8 or more) and to prevent signals on these corridors from going into transition following a pedestrian actuation.

Split times on Farnam Street, Harney Street and 42<sup>nd</sup> Street accommodate pedestrian crossing times via recall.

#### 5.7.4 PLAN 4 OP SPLIT OPTIMIZATION

Split times for minor road through phases at Dodge Street intersections were set to accommodate pedestrian crossing times to prevent signals from going into transition following a pedestrian actuation.

Split times on Farnam Street, Harney Street and 42<sup>nd</sup> Street accommodate pedestrian crossing times via recall.

#### 5.7.4.1 DODGE STREET & FARNAM STREET & HAPPY HOLLOW BOULEVARD

Using a 60-second cycle at Dodge Street & Happy Hollow Boulevard during Plan 4 OP required a lower pedestrian walk of 7 seconds (reduced from 10 seconds) for Phase 6 to allow the split to accommodate the pedestrian time.

Using a 60-second cycle at Farnam Street & Happy Hollow Boulevard during Plan 4 OP required a lower pedestrian walk of 7 seconds (reduced from 11 seconds) for Phase 2 and 6 seconds (reduced from 10 seconds) for Phase 8 to allow the split to accommodate the pedestrian time.







#### 5.7.4.2 FARNAM STREET & SADDLE CREEK ROAD

Using a 60-second cycle at Farnam Street & Saddle Creek Road during Plan 4 OP required a lower pedestrian walk of 7 seconds (reduced from 12 seconds) for Phases 4 and 8. Phase 7 was also changed from Pm+Pt to Permitted during Plan 4 OP.

#### 5.8 OFFSET AND SEQUENCE OPTIMIZATION

Offsets were adjusted and phase sequencing was reviewed and adjusted during the four timing plans. The offset strategies, particularly how corridors were coordinated with adjacent locations, and modified sequences are highlighted in the following sections. A summary of the implemented changes to lead/lag sequencing is shown in Table 5-10.

Table 5-10. Locations with Lagging Left Turn

		Plan 1 MD	Plan 2 AM	Plan 3 PM	Plan 4 OP
Saddle Creek Road & Emile	Existing	Lead	Lead	Lead	Lead
Street – SBL	Implemented	Lag	Lag	No left- turn phase (Permissive only)	Lag

#### 5.8.1 PLAN 1 MD OFFSET AND SEQUENCE OPTIMIZATION

- Dodge Street Balanced stops eastbound and westbound and coordinated with Turner Boulevard
   Dodge Street.
- 42<sup>nd</sup> Street Coordinate northbound 42<sup>nd</sup> Street with 42<sup>nd</sup> Street & Dodge Street.
- Saddle Creek Road Minimized stops on Saddle Creek Road and coordinated with Saddle Creek Road & Leavenworth Street and Saddle Creek Road & Cuming Street intersections.
  - Modified sequence at Saddle Creek Road & Emile Street to lag Phase 7 southbound left (sequence 9).
- Farnam Street west of Saddle Creek Road Coordinated northbound/southbound with Dodge Street
- Farnam Street east of 42<sup>nd</sup> Street Coordinated westbound traffic with Turner Boulevard & Farnam Street.
- Harney Street east of 42<sup>nd</sup> Street Coordinated eastbound traffic with 42<sup>nd</sup> Street & Harney Street.







### 5.8.2 PLAN 2 AM OFFSET AND SEQUENCE OPTIMIZATION

- Dodge Street Continued to favor eastbound progression but looked for way to improve westbound.
- 42<sup>nd</sup> Street Coordinated 42<sup>nd</sup> Street between Emile Street and Farnam Street to have zero or one stop.
- Saddle Creek Road Minimized stops on Saddle Creek Road and coordinated with Saddle Creek Road & Leavenworth Street and Saddle Creek Road & Cuming Street intersections.
  - Modified sequence at Saddle Creek Road & Emile Street to lag Phase 7 southbound left (sequence 9).
- Farnam Street west of Saddle Creek Road Coordinated 50<sup>th</sup> Street and 52<sup>nd</sup> Street together with arrivals from eastbound Dodge Street arriving at 52<sup>nd</sup> Street & Farnam Street on green half of the time due to different cycle lengths.
- Farnam Street east of 42<sup>nd</sup> Street Coordinated westbound traffic between Turner Boulevard & Farnam Street and 42<sup>nd</sup> Street & Farnam Street.
- Harney Street east of 42<sup>nd</sup> Street Coordinated eastbound traffic with 42<sup>nd</sup> Street & Harney Street.

#### 5.8.3 PLAN 3 PM OFFSET AND SEQUENCE OPTIMIZATION

- Dodge Street Continued to favor westbound progression but introduced a stop at 52<sup>nd</sup> Street to reduce the frequency and duration of stops in the eastbound direction.
- 42<sup>nd</sup> Street Coordinated 42<sup>nd</sup> Street between Emile Street and Farnam Street to have zero or one stop. Set 42<sup>nd</sup> Street & Farnam Street offset to allow for westbound progression on Farnam Street between Turner Boulevard and 42<sup>nd</sup> Street.
- Saddle Creek Road Minimized stops on Saddle Creek Road and coordinated with Saddle Creek Road & Leavenworth Street and Saddle Creek Road & Cuming Street intersections.
- Farnam Street west of Saddle Creek Road Coordinated 50<sup>th</sup> Street and 52<sup>nd</sup> Street together with arrivals westbound platoons from 52<sup>nd</sup> Street arriving at Happy Hollow Boulevard on green two out of three cycles due to different cycle lengths.
- Farnam Street east of 42<sup>nd</sup> Street Coordinated westbound traffic between Turner Boulevard & Farnam Street and 42<sup>nd</sup> Street & Farnam Street.
- Harney Street east of 42<sup>nd</sup> Street Coordinated eastbound traffic with 42<sup>nd</sup> Street & Harney Street.







### 5.8.4 PLAN 4 OP OFFSET AND SEQUENCE OPTIMIZATION

- Dodge Street Balanced stops eastbound and westbound and coordinated with Turner Boulevard
   & Dodge Street.
- 42<sup>nd</sup> Street Coordinated northbound 42<sup>nd</sup> Street with 42<sup>nd</sup> Street & Dodge Street and 42<sup>nd</sup> Street
   & Leavenworth Street.
- Saddle Creek Road Minimized stops on Saddle Creek Road and coordinated with Saddle Creek & Leavenworth intersection.
  - Modified sequence at Saddle Creek Road & Emile Street to lag Phase 7 southbound left (sequence 9).
- Farnam Street west of Saddle Creek Road Coordinated northbound/southbound with Dodge Street.
- Farnam Street east of 42<sup>nd</sup> Street Coordinated westbound traffic with Turner Boulevard & Farnam Street.
- Harney Street east of 42<sup>nd</sup> Street Coordinated eastbound traffic with 42<sup>nd</sup> Street & Harney Street.







# 6 IMPLEMENTATION

### 6.1 DATABASE PROGRAMMING

Database programing of the new signal timings started on July 28<sup>th</sup> and was completed on August 3<sup>rd</sup>. The latest database was used to make sure any recent changes made by City staff were included in the base timings. New timings were entered via the MaxView and TrafficView databases.

### **6.2 IMPLEMENTATION DAY**

Implementation of the new signal timings took place the on August 8<sup>th</sup>. Each intersection had an upload done to check for any recent changes since programming began prior to download. Then intersections were observed in-person from the field while the database for that signal was downloaded to the controller to ensure proper operations before moving to the next intersection. HDR and City staff continued to monitor traffic cameras especially during transitions between timing plans.

### 6.3 FINE TUNING

Fine tuning of the new signal timings started immediately following implementation starting with PM peak plan on August 8<sup>th</sup> and was largely completed by August 15<sup>th</sup>. Fine tuning involved making observations during each timing plan throughout the day to verify proper progression of traffic throughout each of the corridors. Intersections were also observed to ensure proper operations of pedestrian signals, LPI, and any other hardware at the intersections. Updates to signal timings were conducted through VPN to the City's controller software. Changes were documented daily and sent to the City. Change log forms were also sent for any changes made to signals with Wapiti controllers.

### 6.4 PUBLIC COMMENT

At this time, no public requests for changes or re-evaluation have been received related to the implemented signal timings.







# 7 PERFORMANCE MEASURES

# 7.1 NETWORK MEASURES OF EFFECTIVENESS

Measures of effectiveness (MOEs) were estimated using Synchro based on the existing and implemented signal timings. **Table 7-1** shows the MOEs broken out by corridor and summed for network-wide performance. The Synchro reports for network MOEs can be found in **Appendix**.





**Table 7-1. Corridor and Network Performance Measures** 

		D	Dodge Street Farnam Street Ha		rney Street		Saddle Creek Road			42nd Street			Network <sup>1</sup>						
		Exist.	Implement	Diff.	Exist.	Implement	Diff.	Exist.	Implement	Diff.	Exist.	Implement	Diff.	Exist.	Implement	Diff.	Exist.	Implement	Diff.
	Total Delay (hr)	72	51	-41%	19	18	-6%	3	3	0%	22	20	-10%	12	12	0%	128	104	-23%
M	Stops (#)	9,143	9,050	-1%	3,078	2,673	-15%	602	524	-15%	2,642	2,535	-4%	1,873	2,065	9%	17,338	16,847	-3%
Ė	Fuel Consumed (gal)	365	349	-5%	76	73	-4%	15	15	0%	105	102	-3%	37	38	3%	598	577	-4%
Plan	Total Travel Time (hr)	264	243	-9%	58	57	-2%	12	12	0%	76	74	-3%	31	31	0%	441	417	-6%
	Performance Index	97.5	76.5	-27%	27.9	25.3	-10%	4.9	4.7	-4%	29.1	26.6	-9%	17.0	18.0	6%	176.4	151.1	-17%
_	Total Delay (hr)	75	72	-4%	18	32	44%	3	3	0%	20	19	-5%	12	14	14%	128	140	9%
AM	Stops (#)	9,371	9,082	-3%	2,984	3,366	11%	531	598	11%	2,110	2,040	-3%	2,115	2,410	12%	17,111	17,496	2%
2 -	Fuel Consumed (gal)	392	387	-1%	75	87	14%	16	17	6%	92	90	-2%	37	39	5%	612	620	1%
Plan	Total Travel Time (hr)	284	280	-1%	56	70	20%	13	13	0%	68	67	-1%	30	32	6%	451	462	2%
	Performance Index	101.3	96.7	-5%	26.2	41.7	37%	4.3	4.6	7%	25.9	24.2	-7%	17.9	20.8	14%	175.6	188.0	7%
_	Total Delay (hr)	126	115	-10%	35	41	15%	5	5	0%	35	35	0%	14	15	7%	215	211	-2%
₽ M	Stops (#)	17,257	14,524	-19%	4,753	4,851	2%	907	806	-13%	4,309	4,072	-6%	2,486	2,494	0%	29,712	26,747	-11%
m -	Fuel Consumed (gal)	554	525	-6%	109	115	5%	23	23	0%	141	139	-1%	46	46	0%	873	848	-3%
Plan	Total Travel Time (hr)	387	376	-3%	85	91	7%	18	18	0%	101	102	1%	38	38	0%	629	625	-1%
	Performance Index	174.1	155.6	-12%	47.7	54.1	12%	7.4	7.4	0%	47.1	46.8	-1%	21.3	22.0	3%	297.6	285.9	-4%
	Total Delay (hr)	50	35	-43%	11	11	0%	2	2	0%	14	11	-27%	7	8	13%	84	67	-25%
9	Stops (#)	6,656	6,598	-1%	2,096	1,804	-16%	398	340	-17%	1,747	2,127	18%	1,297	1,598	19%	12,194	12,467	2%
4- -	Fuel Consumed (gal)	273	262	-4%	52	50	-4%	11	11	0%	74	74	0%	25	27	7%	435	424	-3%
Plan	Total Travel Time (hr)	197	182	-8%	38	39	3%	8	9	11%	54	50	-8%	21	21	0%	318	301	-6%
_	Performance Index	68.2	53.2	-28%	16.7	16.4	-2%	3.1	3.2	3%	19.2	16.5	-16%	11.0	12.3	11%	118.2	101.6	-16%

<sup>&</sup>lt;sup>1</sup> Network MOEs shown in this table are a sum of the MOEs for each corridor and may not exactly match values shown in Appendix.







The network delay values show an improvement from the existing to the implemented during all plans except the AM plan. Reviewing the delays for each corridor during the AM plan, the majority of the network-wide increase in delay came from Farnam Street. While not as large, the Farnam Street delay also increased during the PM plan. Farnam Street is where the majority of LPIs were implemented with a pedestrian recall to enhance pedestrian safety. The recalls and LPIs result in added vehicular delay caused by serving and extending the side street phase during all times and delaying the start of Farnam Street traffic. To determine the amount of impact these safety measures had on vehicular delay, the LPIs and pedestrian recalls were removed from the implemented Synchro models. Table 7-2 shows the MOEs for existing, implemented, and implemented without LPI or pedestrian recalls. These results indicate there would have been a reduction to delay and stops on Farnam Street with the implemented timings during all plans if LPIs and pedestrian recalls had not been added.

Table 7-2. Farnam Street LPI and Recall Sensitivity

		Exist.	lmp.	No LPI or Ped Recall
	Total Delay (hr)	20	20	16
ΔM	Stops(#)	3,078	2,673	2,362
Plan 1 - MD	Fuel Consumed (gal)	76	73	69
an an	Total Travel Time (hr)	58	57	53
<u> </u>	Performance Index	27.9	25.3	20.7
_	Total Delay (hr)	18	28	17
₽	Stops(#)	2,984	3,366	2,659
Plan 2 - AM	Fuel Consumed (gal)	75	87	71
lan	Total Travel Time (hr)	56	70	53
4	Performance Index	26.2	41.7	22.7
_	Total Delay (hr)	26	31	23
Plan 3 - PM	Stops(#)	4,753	4,851	4,150
ω <sub>-</sub>	Fuel Consumed (gal)	109	115	101
lan	Total Travel Time (hr)	85	91	77
-	Performance Index	47.7	54.1	38.2
	Total Delay (hr)	15	17	15
Q.	Stops(#)	2,096	1,804	1,701
Plan 4 - OP	Fuel Consumed (gal)	52	50	49
lar	Total Travel Time (hr)	38	39	37
	Performance Index	16.7	16.4	14.2







## 7.2 INTERSECTION OPERATIONS

Intersection operations were also compared between the existing and implemented signal timings. Table 7-3 summarizes the number of intersections where delay decreased, was unchanged, increased by less than five seconds, and increased by more than five seconds. Existing and implemented Synchro reports for individual intersections can be found in Appendix.

**Table 7-3. Intersection Delay Change Summary** 

Number of Intersections Where:	Plan 1 MD	Plan 2 AM	Plan 3 PM	Plan 4 OP
Delay Decreased	26	18	17	22
Delay Unchanged	0	1	0	0
Delay Increased <=5 sec/veh	11	15	18	15
Delay Increased >5 sec/veh	0	3	2	0

Further review was completed for intersections that had an increase of more than five seconds of delay. The intersections that had delay increases during a plan are shown in Table 7-4.

**Table 7-4. Intersections with Delay Increase Over Five Seconds** 

Intersection	Plan	Existing Delay and LOS	Implemented Delay and LOS	Difference
32nd St & Farnam St	Plan 2 AM	2.4 - A	17.0 - B	14.6
33rd St & Farnam St	Plan 2 AM	7.2 - A	24.8 - C	17.6
33rd St & Farnam St	Plan 3 PM	10.9 - B	27.1 - C	16.2
34th St & Farnam St	Plan 2 AM	2.7 - A	7.8 - A	5.1
Saddle Creek Rd & Davenport St	Plan 3 PM	8.4 - A	16.1 - B	7.7

Similar to the results shown in the network performance measures, most of the locations that saw a larger increase in delay were locations where LPI and pedestrian recalls were implemented to enhance awareness of pedestrians crossing the street. At these locations, if the LPI and pedestrian recalls are removed, the resulting delay is similar to existing. Below is a list of reasons for the increased delay at each intersection listed in Table 7-4:

- 32<sup>nd</sup> Street & Farnam Street (AM): Implementation of LPI and pedestrian recall.
- 33<sup>rd</sup> Street & Farnam Street (AM): Implementation of LPI and pedestrian recall.
- 33<sup>rd</sup> Street & Farnam Street (PM): Implementation of LPI and pedestrian recall.
- 34<sup>th</sup> Street & Farnam Street (AM): Implementation of LPI and pedestrian recall.
- Saddle Creek Road & Davenport Street (PM): This intersection was changed to a set stop during the PM for better overall progression on Saddle Creek Road.







# 7.3 DODGE STREET TRAVEL TIME RUN PERFORMANCE

Travel time runs were conducted during five periods as detailed in the **Travel Time Runs** section of Chapter 3. The average travel time run data from the existing and implemented travel time runs were plotted for comparison with free flow travel conditions based on the posted speed limit. The difference between the lines illustrates the difference in travel time between scenarios. All five graphs and a table with the average travel time and delay for each scenario can be found in **Figure 7-1** on the following page. Time-space diagram plots for the existing and implemented for each time period can be found in **Appendix**.

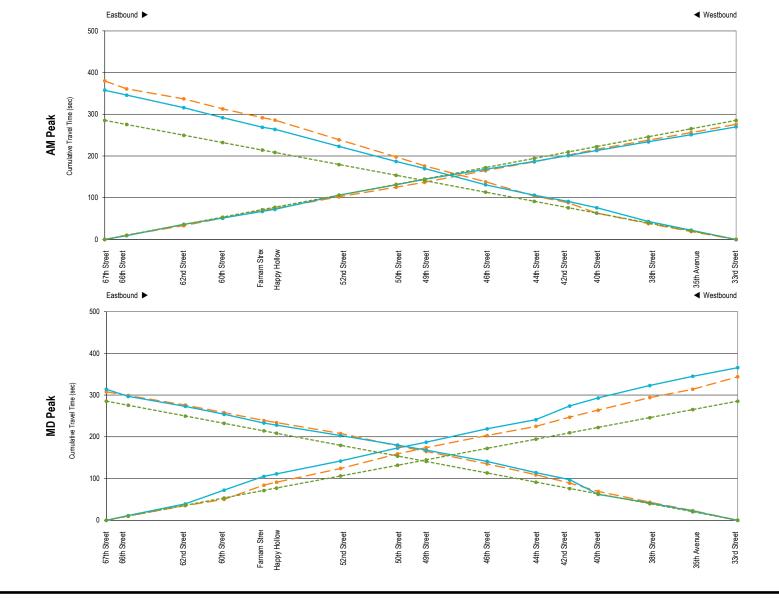


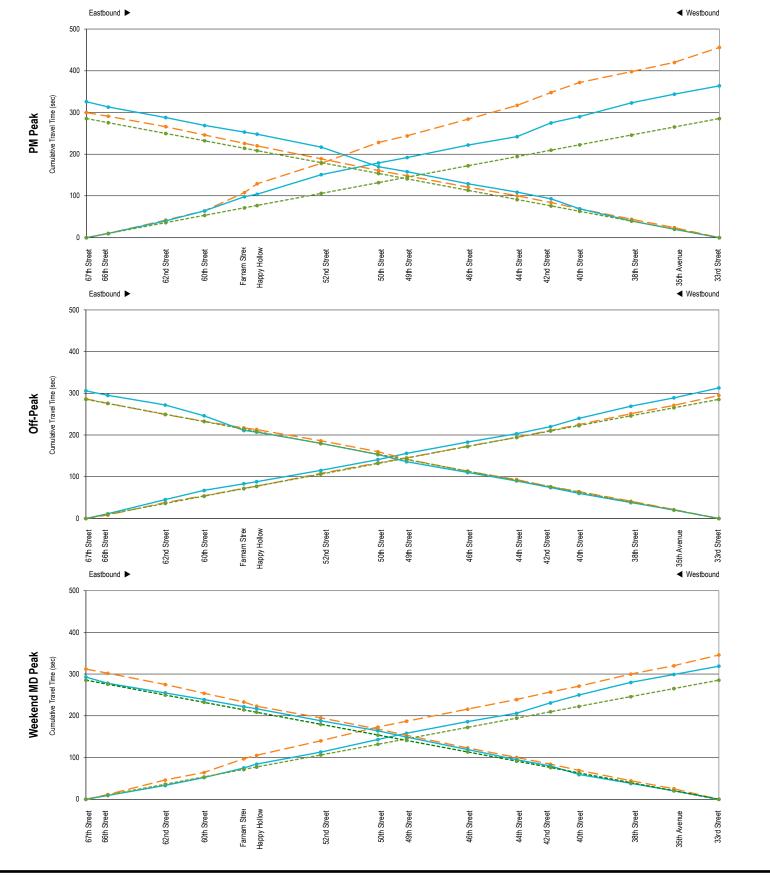
Average Total Travel Time & Delay

Dodge Street: 2.8 miles

		AM Peak		MD Peak		PM Peak		Off-Peak		Weekend MD Peak	
		Travel Time (s)	Delay (s)								
р	Existing	276	-12	344	56	456	168	295	8	346	58
ē	Implemented	270	-18	366	78	364	77	313	25	319	31
	Difference	-6	ô	22		-92		18		-27	
	% Difference	-2.2%	50.0%	6.4%	39.3%	-20.2%	-54.8%	6.1%	225.0%	-7.8%	-46.6%
рu	Existing	380	93	308	21	300	13	287	-1	312	24
noc	Implemented	358	70	314	27	326	38	306	18	293	5
stb	Implemented Difference	-22		6		26		19		-19	
š	% Difference	-5.8%	-23.7%	1.9%	28.6%	8.7%	200.0%	6.6%	-1900.0%	-6.1%	-79.2%

Eastbound: 67th Street to 33rd Street Westbound: 33rd Street to 67th Street













The results from the eastbound AM travel time runs showed that the implemented timings remained nearly the same as the existing with no stops. . In the westbound direction, there was an improvement of about 22 seconds and two fewer stops on average with the implemented timings.

During the MD, travel times in the eastbound direction increased by 22 seconds from the existing to the implemented timings. In the westbound direction, it stayed nearly the same with a slight increase of six seconds. While a shorter cycle length on Dodge Street led to longer travel times and more delay for Dodge Street traffic, the overall delay (including side street delay) in Table 7-1 shows a decrease in delay. This means that the reduction in side street delay was greater than the increase in Dodge Street delay.

The PM travel time results show an increase in travel time of 26 seconds in the westbound direction, but a decrease of 92 seconds in the westbound direction. On average, the westbound direction added one stop at 52<sup>nd</sup> Street compared to the existing. The eastbound direction averaged three or four stops instead of the five to seven stops with the existing signal timings.

OP travel times increased by about 20 seconds in both directions from the existing to the implemented timings. Similar to the MD timings, the cycle length was decreased on Dodge Street to better serve traffic on the side streets. **Table 7-1** shows a decrease in overall delay on Dodge Street during the OP plan once side street delay is taken into account.

The weekend travel time runs were conducted during the portion of the day using the MD timing plan. However, there was less side street traffic compared to weekday MD travel time runs so Dodge Street traffic was stopped less frequently and the travel times decreased in both directions. The travel time decreased by 27 seconds in the eastbound direction and 19 seconds in the westbound direction.







# 8 BENEFIT COST ANALYSIS

A benefit-cost analysis was conducted using the network performance measures from the previous chapter. The analysis used the estimated delay, fuel consumption, CO emissions, NOx emissions, and VOC emissions to estimate the overall benefits of the project. An estimation of benefits from crash reduction was also made using US Department of Transportation (USDOT) guidelines and the number of crashes per year citywide. Appendix shows the estimated benefits in each of the categories.

The City has developed a methodology for monetizing project benefits over five years using USDOT guidelines. The total project benefits are estimated to be \$3.8 million. The total costs for the project were \$131,972. This results in a benefit-cost ratio of 29:1 over a five-year period.

**Table 8-1. Anticipated Five Year Project Benefits** 

Performance Measure	Project Benefit	Present Value
Delay Reduction	30,295 hours	\$752,835
Fuel Consumption Reduction	47,518 gallons	\$161,474
Emission Reduction	424 tons	\$32,467
Crash Reduction	53 crashes	\$2,893,882







# 9 SAFETY AND OPERATIONAL RECOMMENDATIONS

Observations for safety and operational deficiencies were noted throughout the project and the following recommendations are made with supporting observations.

- Perform tree maintenance on Farnam Street at the following locations:
  - Farnam Street Dodge Street to Saddle Creek Road
    - Street signs are partially blocked in both directions at multiple locations throughout.
  - Farnam Street Westbound approaching Turner Boulevard
    - Only one of three traffic signal heads are visible when approaching intersection due to overgrown trees on the northwest corner.
- Review operations on north leg of 44<sup>th</sup> Street & Dodge Street for potential one-way operation.
  - The north leg of intersection operates with two-way traffic despite only being wide enough for a single vehicle. There is no signage at or approaching the intersection indicating that it is one-way.
- Review need for existing school crossing signs at 35<sup>th</sup> Street & Dodge Street.
  - School crossing signs are installed on the signal mast arm on all four approaches. This
    intersection is within the attendance area for Gifford Park Elementary, but there may be
    closer crossings of Dodge Street that are more appropriate.
- Consider restricting right-turn-on-red for westbound traffic at 42<sup>nd</sup> Street & Farnam Street.
  - The sight distance for westbound right-turn traffic is poor at the intersection due to a
    retaining wall close to the roadway making it difficult to see southbound traffic. The City
    should review crashes at this location during consideration.
- Consider restricting right-turn-on-red for southbound traffic at 50<sup>th</sup> Street & Dodge Street.
  - The sight distance for southbound right-turn traffic is poor at the intersection due to buildings close to the roadway making it difficult to see westbound traffic. The City should review crashes at this location during consideration.
- Consider restricting right-turn-on-red for southbound traffic at 52<sup>nd</sup> Street & Dodge Street.
  - The sight distance for southbound right-turn traffic is poor at the intersection due to a
    retaining wall close to the roadway and vertical grade making it difficult to see westbound
    traffic. The City should review crashes at this location during consideration.
- Add louvers or optically programmed signal heads for the eastbound approach of Turner Boulevard & Douglas Street
  - Eastbound Dodge Street traffic curves south at 31<sup>st</sup> Street to create a one-way pair with Douglas Street. At the intersection with Turner Boulevard, the eastbound approaching traffic can see both the eastbound signal heads and the southbound signal heads. This signal was reconstructed in 2022.







# **APPENDIX**

